



# **613 Helena Street Town of Fort Erie Transportation Impact Study Update**

Paradigm Transportation Solutions Limited

December 2021  
20371



# Project Summary



## Project Number

210371

## Date: December 2021

Version 1.0.0

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# 613 Helena Street Town of Fort Erie Transportation Impact Study Update



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# Executive Summary

## Content

SS Fort Erie Inc. retained Paradigm Transportation Solutions Limited (Paradigm) to conduct this Transportation Impact Study (TIS), for a proposed townhouse development located at 613 Helena Street in the Town of Fort Erie.

This study determines the impacts of the development traffic on the surrounding road network and identifies the recommended improvements to accommodate the site generated traffic.

## Development Concept

The subject site is located at 613 Helena Street in the Town of Fort Erie. The development concept includes 116 townhouse units.

Development and occupancy of the site is estimated to occur by Year 2024.

Vehicle access to the site is proposed via two new accesses to Helena Street.

## Conclusions

Based on the investigations carried out, it is concluded that:

- ▶ **Base Year Traffic Conditions:** The study area intersections are operating at acceptable levels of service and all movements are well within capacity during the weekday AM and PM peak hours. No critical movements are noted.
- ▶ **Site Trip Generation:** The proposed development is estimated to generate approximately 55 AM peak hour vehicle trips and 67 PM peak hour vehicle trips.
- ▶ **Background Traffic Conditions:** The study area intersections are forecast to operate at acceptable levels of service and all movements are well within capacity during the weekday AM and PM peak hours. No critical movements are noted.
- ▶ **Total Traffic Conditions:** The study area intersections are forecast to operate at acceptable levels of service and all movements are well within capacity during the weekday AM and PM peak hours. No critical movements are noted.



The site driveways are forecast to operate in the LOS A-B range with a v/c of less than 0.05 and 95<sup>th</sup> percentile queue lengths of less than 15 m.

- ▶ **Remedial Measures:** No improvements to the existing and proposed form of traffic control are required at the study area intersections.

Northbound left-turn lanes on Helena Street at the site driveways are not warranted based on forecast traffic volumes.

## Recommendation

Based on the findings of this study, it is recommended that the site driveway approaches to Helena Street operate as unsignalized intersections.



# Contents

<b>1</b>	<b>Introduction .....</b>	<b>1</b>
<b>2</b>	<b>Existing Conditions.....</b>	<b>3</b>
2.1	Road Network .....	3
2.2	Active Transportation .....	5
2.3	Transit Network .....	7
2.4	Traffic Volumes .....	9
2.4.1	Turning Movement Count Data .....	9
2.4.2	ATR Count.....	11
2.5	Traffic Operations .....	12
<b>3</b>	<b>Development Concept.....</b>	<b>15</b>
3.1	Description.....	15
3.2	Site Trip Generation .....	17
3.3	Sight Distance .....	19
<b>4</b>	<b>Future Traffic Conditions.....</b>	<b>21</b>
4.1	Forecast Traffic .....	21
4.2	Background Traffic .....	24
4.3	Total Traffic.....	26
<b>5</b>	<b>Remedial Measures .....</b>	<b>28</b>
5.1	Auxiliary Left-Turn Lanes .....	28
<b>6</b>	<b>Conclusions and Recommendations .....</b>	<b>29</b>
6.1	Conclusions.....	29
6.2	Recommendations .....	29

# Appendices

<b>Appendix A</b>	<b>Pre-Study Consultation</b>
<b>Appendix B</b>	<b>Existing Data</b>
<b>Appendix C</b>	<b>Base Year Traffic Operations</b>
<b>Appendix D</b>	<b>Background Traffic Operations</b>
<b>Appendix E</b>	<b>Total Traffic Operations</b>
<b>Appendix F</b>	<b>Left-Turn Lane Warrants</b>



## Figures

<b>Figure 1.1:</b>	<b>Site Location .....</b>	<b>2</b>
<b>Figure 2.1:</b>	<b>Existing Lane Configuration &amp; Traffic Control .....</b>	<b>4</b>
<b>Figure 2.2:</b>	<b>Future Cycling Network .....</b>	<b>6</b>
<b>Figure 2.3:</b>	<b>Existing Transit Network .....</b>	<b>8</b>
<b>Figure 2.4:</b>	<b>Base Year Traffic Volumes .....</b>	<b>10</b>
<b>Figure 3.1:</b>	<b>Site Concept Plan .....</b>	<b>16</b>
<b>Figure 3.2:</b>	<b>Site Generated Traffic .....</b>	<b>18</b>
<b>Figure 4.1:</b>	<b>Forecast Background Traffic Volumes .....</b>	<b>22</b>
<b>Figure 4.2:</b>	<b>Forecast Total Traffic Volumes .....</b>	<b>23</b>

## Tables

<b>Table 2.1:</b>	<b>Existing Turning Movement Count Data.....</b>	<b>9</b>
<b>Table 2.2:</b>	<b>ATR Summary .....</b>	<b>11</b>
<b>Table 2.3:</b>	<b>Base Year Traffic Operations .....</b>	<b>14</b>
<b>Table 3.1:</b>	<b>Site Generated Traffic .....</b>	<b>17</b>
<b>Table 3.2:</b>	<b>Estimated Trip Distribution.....</b>	<b>17</b>
<b>Table 3.3:</b>	<b>TAC Guide Sight Distances .....</b>	<b>19</b>
<b>Table 4.1:</b>	<b>Background Traffic Operations .....</b>	<b>25</b>
<b>Table 4.2:</b>	<b>Total Traffic Operations .....</b>	<b>27</b>



# 1 Introduction

SS Fort Erie Inc. retained Paradigm Transportation Solutions Limited (Paradigm) to conduct this Transportation Impact Study (TIS), for a proposed townhouse development located at 613 Helena Street in the Town of Fort Erie.

**Figure 1.1** illustrates the location of the subject site.

The scope of the study includes:

- ▶ An assessment of the current traffic and site conditions within the study area;
- ▶ Estimates of background traffic growth;
- ▶ Estimates of additional traffic generated by the subject site;
- ▶ Analyses of the impact of the future traffic on the surrounding road network; and
- ▶ Recommend remedial measures to mitigate the additional traffic on the study area road network, if required.

**Appendix A** contains the pre-study consultation correspondence with the Town of Fort Erie and Niagara Region. The study has been conducted in general accordance with the Niagara Region Traffic Impact Study Guidelines<sup>1</sup>.

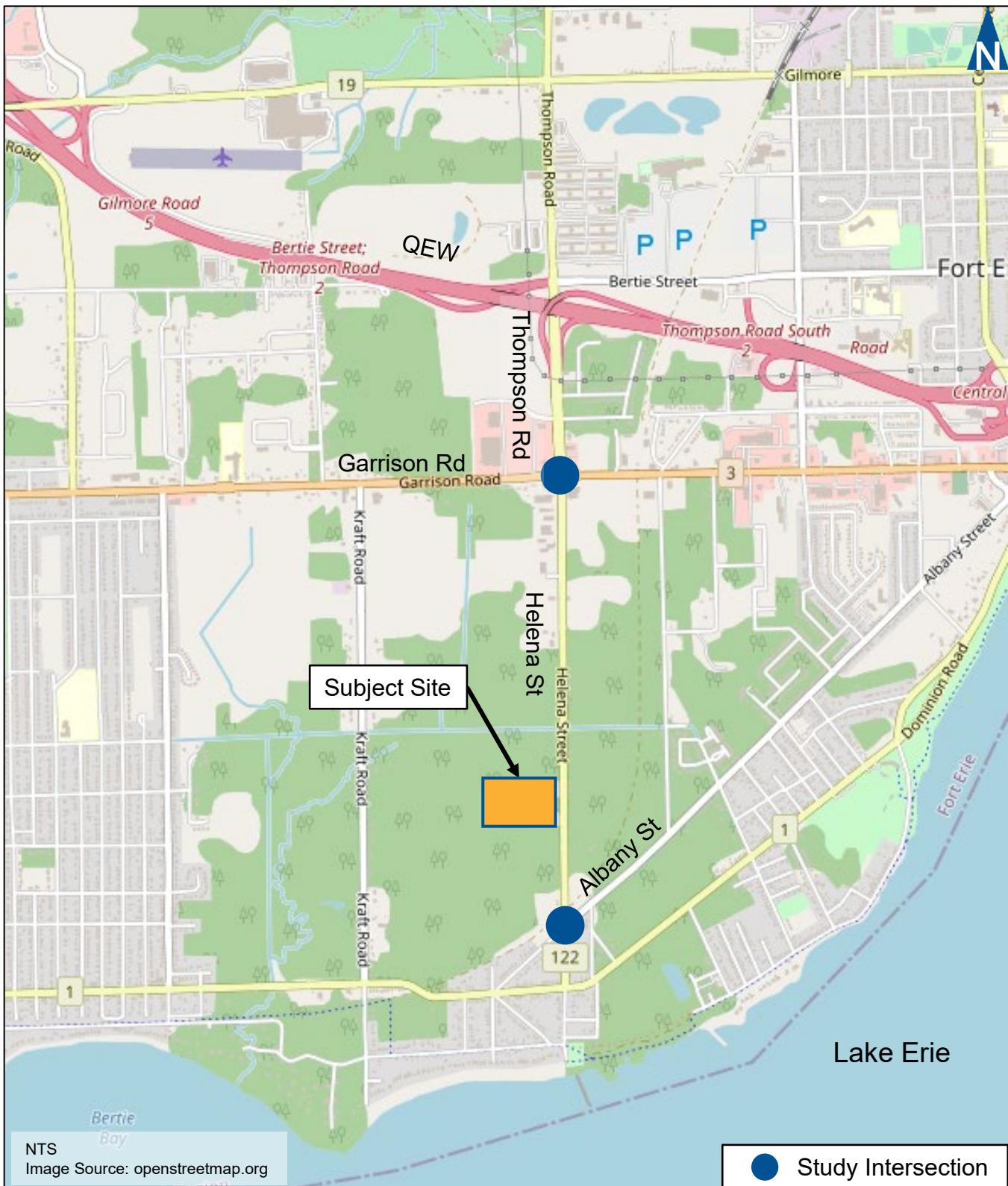
The study area intersections assessed include:

- ▶ Helena Street / Thompson Road at Garrison Road (signalized);
- ▶ Helena Street at Washington Road / Albany Street (unsignalized); and
- ▶ Two proposed site driveways to Helena Street.

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<sup>1</sup> *Guidelines for Transportation Impact Studies*, Niagara Region, May 2021





## Site Location

## 2 Existing Conditions

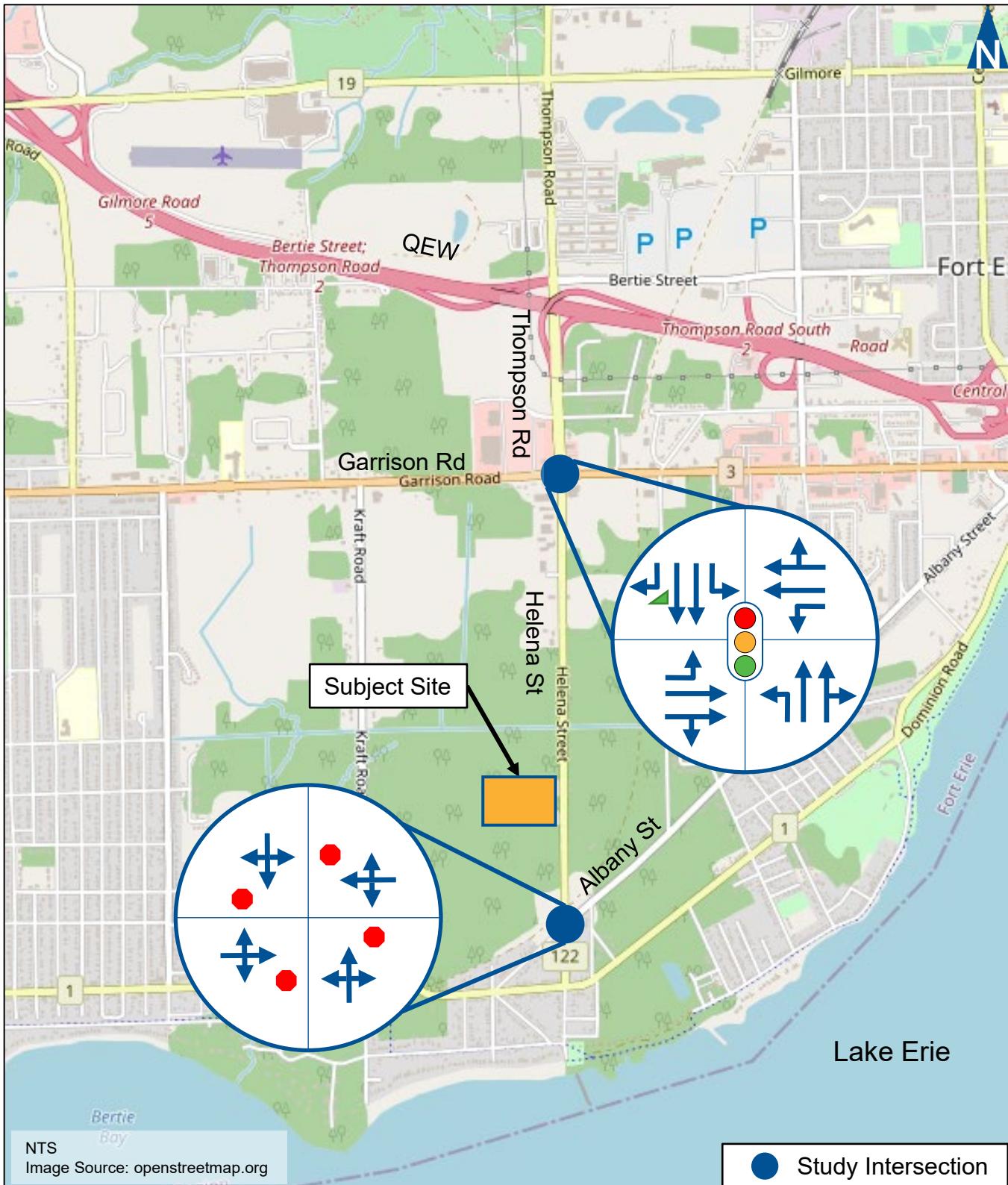
### 2.1 Road Network

The characteristics of the roadways within the study area are described as follows:

- ▶ **Garrison Road** (Regional Road 3) is an east / west roadway with 4-lanes and a posted speed limit of 60 km/h. Sidewalks are provided on both sides of the roadway within the study area. The intersection with Thompson Road / Helena Street is signalized.
- ▶ **Helena Street** (Regional Road 122) is a north / south roadway. The roadway has 2-lane and widens to 4-lanes near the Garrison Road intersection. There is a posted speed limit of 70 km/h north of the subject site, a transition to 50 km/h south of the subject site. No sidewalks are provided within the study area. The intersection with Washington Road / Albany Street is all-way stop controlled.
- ▶ **Thompson Road** (Regional Road 122) is a north / south roadway with 4-lanes and a posted speed limit of 70 km/h. There are sidewalks on both sides of the roadway.
- ▶ **Washington Road / Albany Street** is an east / west roadway with 2-lanes and a posted speed limit of 50 km/h. No sidewalks are present within the study area.

**Figure 2.1** illustrates the existing lane configuration and traffic control at the study area intersections.





## Existing Lane Configuration & Traffic Control

## 2.2 Active Transportation

Sidewalks are provided on both sides of Garrison Road and Thompson Road. No sidewalks are present anywhere else in the study area.

Marked crosswalks and pedestrian push buttons are provided on all approaches at the Helena Street / Thompson Road and Garrison Road intersection. No crosswalks are present at the Helena Street and Washington Road / Albany Street intersection.

No existing cycling infrastructure is identified within the study area.

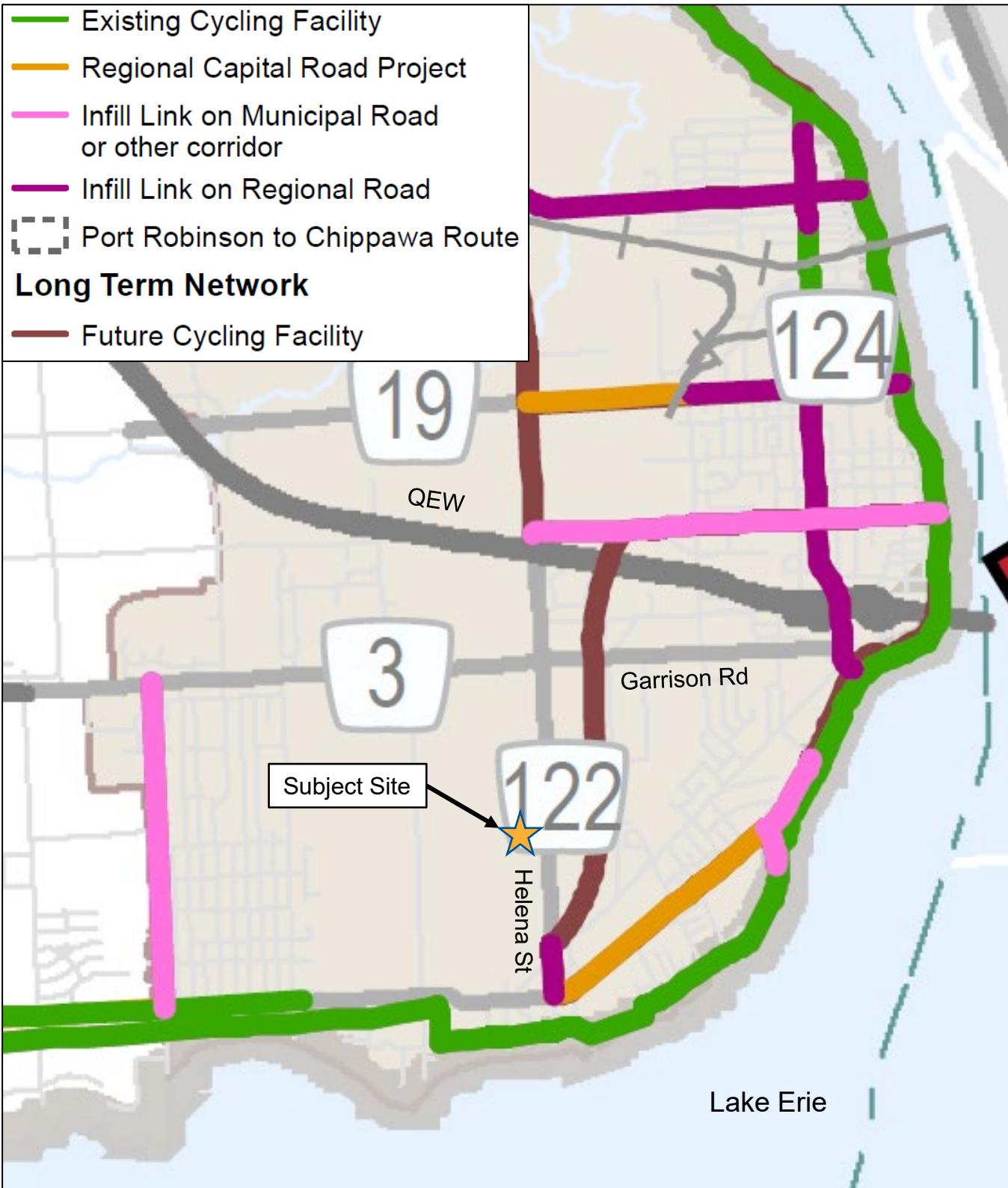
A future cycling facility<sup>2</sup> is proposed to formalize an existing trail that crosses Helena Street approximately 70 metres north of Washington Road / Albany Street, runs between Helena Street and Alfred Street, then crosses Garrison Road.

**Figure 2.2** illustrates the future cycling network.

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<sup>2</sup> *Strategic Cycling Network Development Technical Paper*, IBI Group, June 2017.





## Future Cycling Network

## 2.3 Transit Network

Fort Erie Transit (FET) operates the public transit system in the Town of Fort Erie. **Figure 2.3** illustrates the existing transit network.

The closest transit stop is located approximately 650 m south of the subject site on Dominion Road at Helena Street. However, as no sidewalks are present on Helena Street, transit usage is expected to be low. The road authority should consider adding a sidewalk on Helena Street.

Fort Erie Accessible Specialized Transit (FAST) provides curb-to-curb transportation services within Fort Erie to people who, due to a mobility challenge, would be physically unable to board the conventional transit buses or walk 175 m (600 ft). Service is available Monday to Saturday from 6:00 AM to 9:00 PM but is unavailable Sunday or statutory holidays.





## 2.4 Traffic Volumes

### 2.4.1 Turning Movement Count Data

**Table 2.1** summarizes the location and date of the existing available Turning Movement Count (TMC) data used in the analysis. Paradigm collected TMC data at the Helena Street and Washington Road / Albany Street intersection and Niagara Region provided the data for the Helena Street / Thompson Road and Garrison Road intersection.

**Appendix B** contains the detailed existing count data and signal timing data for the study area intersection.

**TABLE 2.1: EXISTING TURNING MOVEMENT COUNT DATA**

Intersection	TMC Year
Helena St at Washington Rd / Albany St	January 2018
Helena St / Thompson Rd at Garrison Rd	May 2019

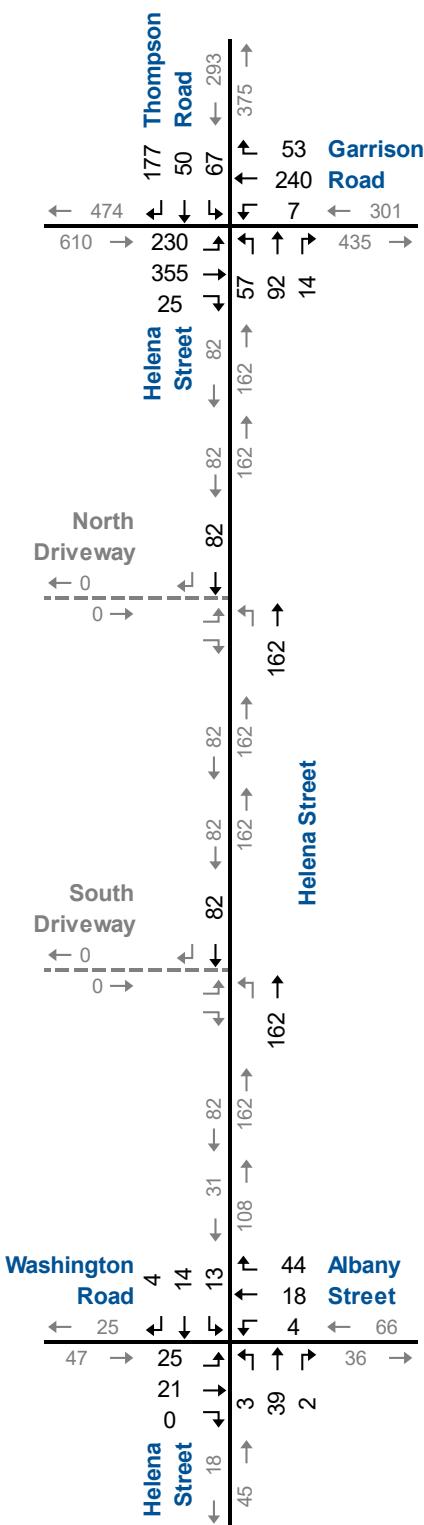
The TMC data was factored to a Year 2021 base year condition by applying a growth rate of 2.0% per annum. The methodology used to develop the base year forecasts was confirmed with the Town and Region during pre-study consultations.

**Figure 2.4** illustrates the base year weekday AM and PM peak hour volumes.

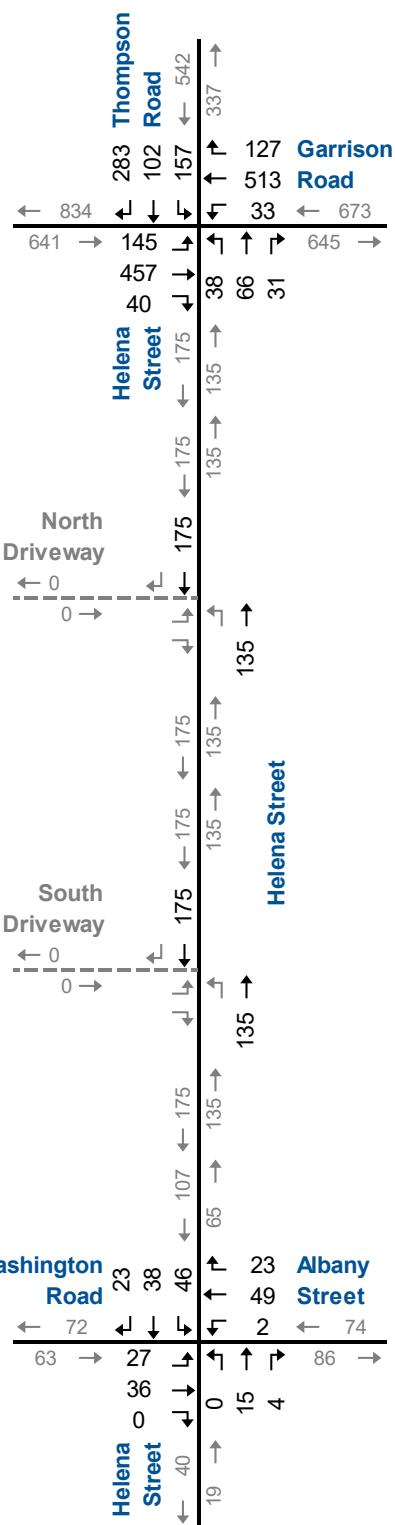




### AM Peak Hour



### PM Peak Hour



## Base Year Traffic Volumes



## 2.4.2 ATR Count

Pyramid Traffic Inc. undertook a 24-hour Automatic Traffic Recorder (ATR) count on Helena Street across the site's frontage. The count data provides the volume of vehicles, the speed, and the classification.

Traffic data was recorded on 28 July 2021 from midnight to 11:59 PM. The traffic count data was collected in 15-minute increments over a 24-hour period.

**Appendix B** contains the 24-Hour Speed / Volume / Classification Count data.

**Table 2.2** summarizes the 2021 ATR count data and compares it to the 2018 ATR count data previously collected in the 2018 TIS<sup>3</sup>. The following is noted:

- ▶ The average 2021 speed is approximately 5 km/h higher than in 2018.
- ▶ The 2021 85<sup>th</sup> percentile speed is approximately 10 km/h higher than in 2018.
- ▶ The overall number of vehicles increased in 2021, however there was a significant decrease in the number of heavy vehicles.

**TABLE 2.2: ATR SUMMARY**

Attribute	Metric	2021 Data			Change from 2018		
		NB	SB	2-Way	NB	SB	2-Way
Speed (km/h)	Average	73	74	-	4	6	-
	85th Percentile	87	86	-	10	9	-
Volume	Recorded Vehicles	1136	1283	2419	112	312	424
Classification	Passenger Vehicles	1094	1235	2329	295	430	725
	Heavy Vehicles	7	14	21	-200	-145	-345
	Trucks / Buses	27	26	53	15	23	38
	Tractor Tailers	8	8	16	2	4	6

<sup>3</sup> 613 Helena Street TIS, Paradigm Transportation Solutions, April 2018.



## 2.5 Traffic Operations

Intersection level of service (LOS) is a recognized method of quantifying the efficiency of traffic flow at intersections. It is based on the delay experienced by individual vehicles executing the various movements. The delay is related to the number of vehicles wanting to make a movement, compared to the estimated capacity for that movement. The capacity is based on several criteria related to the opposing traffic flows. The highest possible rating is LOS A, under which the average total delay is equal or less than 10.0 seconds per vehicle. When the average delay exceeds 80 seconds at signalized intersections (50 seconds at unsignalized), the movement is considered to have a LOS F and remedial measures are usually implemented if they are feasible.

The operations of the intersections in the study area were evaluated under existing conditions using Synchro 10. The intersection analysis considered three separate measures of performance:

- ▶ The LOS for each turning movement. LOS is based on the average control delay per vehicle and was evaluated using HCM 2000 procedures;
- ▶ The volume to capacity ratio for each intersection was evaluated using HCM 2000 procedures; and
- ▶ 95th percentile queue length (m) was evaluated using SimTraffic.

Under the Region's TIS Guidelines, the following criteria indicate critical conditions and signify that mitigation measures may need to be considered:

- ▶ Volume to Capacity ratios ( $v/c$ ) for through or shared through/turning movements that exceed 0.85 at a signalized intersection;
- ▶  $v/c$  ratios for exclusive turning movements that exceed 0.90 at a signalized intersection;
- ▶ The 95<sup>th</sup> percentile queues for an individual movement are projected to exceed available turning lane storage; and
- ▶ LOS, based on average delay per vehicle on individual movements, operate at LOS D or worse for an unsignalized intersection.



**Table 2.3** summarizes the level of service conditions. No critical movements are noted.

**Appendix C** contains the detailed Synchro 10 reports.



**TABLE 2.3: BASE YEAR TRAFFIC OPERATIONS**

Analysis Period	Intersection	Control Type	MOE	Direction / Movement / Approach																Overall	
				Eastbound				Westbound				Northbound				Southbound					
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach		
AM Peak Hour	Garrison Rd & Helena St / Thompson Rd	TCS	LOS	A	A	>	A 7	B	B	>	B 14	B	B	>	B 16	B	B	B	B 16 0.39		
			Delay	7	7	>		15	14	>		17	16	>		17	16	16	0.14		
PM Peak Hour	Helena St & Washington Rd / Albany St	AWSC	V/C	0.45	0.24	>	A 8	0.03	0.29	>	A 7	0.24	0.14	>	A 8	0.27	0.08	0.14	A 8		
			Q	37	24	>		6	32	>		26	19	>		24	21	2			
	Garrison Rd & Helena St / Thompson Rd	TCS	Ex	120	-	>	A 9	110	-	>	B 16	35	-	>	B 19	70	-	-	B 15 0.51		
			Avail.	84	-	>		104	-	>		9	-	>		46	-	-			
	Helena St & Washington Rd / Albany St	AWSC	LOS	<	A	>	A 8	<	A	>	A 7	<	A	>	A 8	<	A	>	A 8		
			Delay	<	8	>		<	7	>		<	8	>		<	8	>			
	Garrison Rd & Helena St / Thompson Rd	TCS	V/C	<	0.06	>	A 9	<	0.08	>	B 16	<	0.06	>	B 19	<	0.04	>	B 15 0.51		
			Q	<	12	>		<	12	>		<	7	>		<	7	>			
	Helena St & Washington Rd / Albany St	AWSC	Ex	120	-	>	A 8	110	-	>	A 8	35	-	>	A 8	70	-	-	A 8		
			Avail.	89	-	>		97	-	>		16	-	>		31	-	-			

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

D.Util - Degree of Utilization

Q - 95th Percentile Queue Length

Ex. - Existing Available Storage

Avail. - Available Storage

&lt; - Shared Left-turn

&gt; - Shared Right-turn

TCS - Traffic Control Signal

AWSC - All-Way Stop Control

TWSC - Two-Way Stop Control



## 3 Development Concept

### 3.1 Description

The subject site is located at 613 Helena Street in the Town of Fort Erie. The development concept includes 116 townhouse units.

Development and occupancy of the site is estimated to occur by Year 2024.

Vehicle access to the site is proposed via two new accesses to Helena Street.

**Figure 3.1** illustrates the site concept plan.



**Figure 3.1**

## Site Concept Plan



## 3.2 Site Trip Generation

ITE Trip Generation<sup>4</sup> data was used to estimate the vehicular trip generation. LUC 220 - Multifamily Housing (Low-Rise) was used to estimate the site's trip generation using regression equations. **Table 3.1** summarizes the estimated trip generation. To remain conservative, no modal split adjustments have been applied to the trip generation estimate to account for active transportation or transit-oriented trips.

The subject site is forecast to generate approximately 55 and 67 new vehicle trips during the AM and PM peak hours, respectively.

**TABLE 3.1: SITE GENERATED TRAFFIC**

Land Use	AM Peak Hour			PM Peak Hour		
	In	Out	Total	In	Out	Total
LUC 220 – 166 Units	13	42	55	42	25	67
<b>Total Generation</b>	13	42	55	42	25	67

$$\text{AM} \mid \ln(T) = 0.95 \ln(X) - 0.51, \text{PM} \mid \ln(T) = 0.89 \ln(x) - 0.02$$

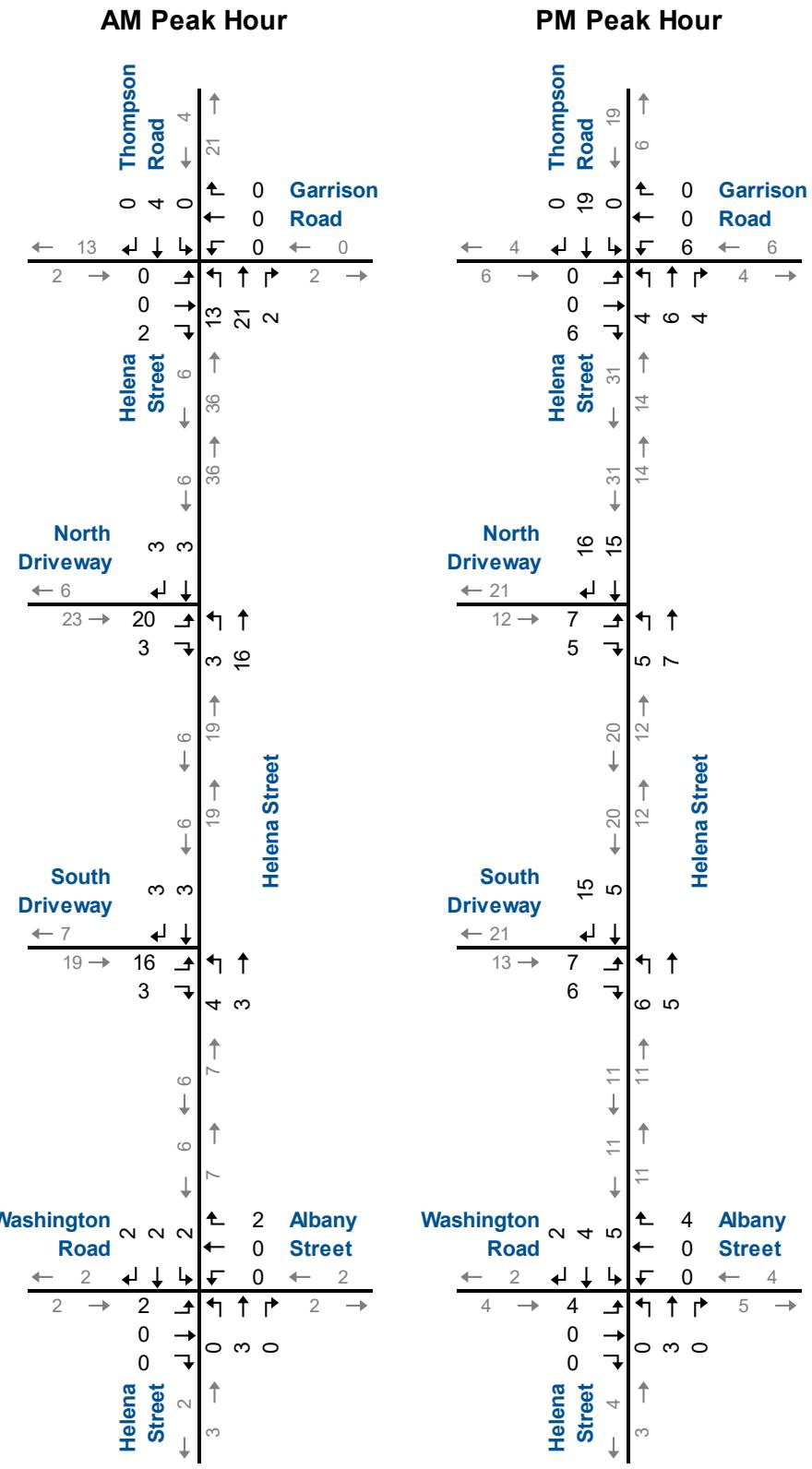
**Table 3.2** summarizes the estimated trip distribution. The estimated distribution is based on existing conditions as the nearby area is primarily residential. **Figure 3.2** illustrates the site generated traffic.

**TABLE 3.2:ESTIMATED TRIP DISTRIBUTION**

Origin/Destination	AM Peak Hour		PM Peak Hour	
	In	Out	In	Out
North via Thompson Road	30%	50%	45%	25%
East via Garrison Road	5%	5%	15%	15%
West via Garrison Road	15%	30%	15%	15%
South via Helena Street	20%	5%	5%	15%
East via Albany Street	20%	5%	10%	20%
West via Washington Road	10%	5%	10%	10%
<b>Total</b>	100%	100%	100%	100%

<sup>4</sup> *Trip Generation Tenth Edition*, Institute of Transportation Engineers, Washington D.C., 2017.





### 3.3 Sight Distance

The available sight distance at the proposed driveway locations have been assessed based on the methodology outlined in the Transportation Association of Canada (TAC) *Geometric Design Guide for Canadian Roads*<sup>5</sup> ("TAC Guide"). The following parameters have been referenced in the assessment:

- ▶ Object Height (vehicle tail or brake light) – 0.60 metres;
- ▶ Driver Eye Height – 1.08 metres; and
- ▶ Top of Car – 1.30 metres.

The sight distance requirements for the proposed driveway locations were determined based on a design speed of 90 km/h, which is 20 km/h above the posted speed limit Helena Street. **Table 3.3** summarizes the sight distance measurements set out in the TAC Guide for level (flat) roadways applicable to this analysis.

**TABLE 3.3: TAC GUIDE SIGHT DISTANCES**

Sight Distance	Distance (m) (90 km/h Design Speed)
Minimum Stopping Sight Distance <sup>6</sup> <i>Driver approaching the site driveway.</i>	160 m
Intersection Sight Distance (Left Turn from Stop) <sup>7</sup> <i>Driver looks to the right before turning left to exit the site.</i>	190 m
Intersection Sight Distance (Right Turn from Stop) <sup>8</sup> <i>Driver looks to the left before turning right to exit the site.</i>	165 m

Paradigm staff completed a site visit on 05 August 2021 to measure the existing sight distances at the proposed driveway locations. The

<sup>5</sup> Transportation Association of Canada. *Geometric Design Guide for Canadian Roads*. June 2017.

<sup>6</sup> Ibid. *Table 2.5.2: Stopping Sight Distance on Level Roadways for Automobiles*.

<sup>7</sup> Ibid. *Table 9.9.4: Design Intersection Sight Distance – Case B1, Left Turn from Stop*.

<sup>8</sup> Ibid. *Table 9.9.6: Design Intersection Sight Distance – Case B2, Right Turn from Stop*.



sight distances at the proposed driveway locations are greater than 300 m and exceed the sight distances identified in the TAC Guide for a design speed of 90 km/h.



## 4 Future Traffic Conditions

The assessment of future conditions in this section includes the following components:

- ▶ Future background traffic estimates;
- ▶ Level of service analysis for background traffic (pre-development);
- ▶ Future total traffic estimates; and
- ▶ Level of service analysis for total traffic (post-development).

### 4.1 Forecast Traffic

A five-year horizon (Year 2026) from TIS commission has been assessed. The likely future traffic volumes near the subject site are estimated to consist of:

- ▶ Increased non-site traffic (generalized background traffic growth). A growth rate of 2.0% per annum was confirmed by the Region during pre-study consultations; and
- ▶ Traffic generated by the proposed development.

It is noted no other area background developments were identified by Region staff for inclusion within the traffic forecasts.

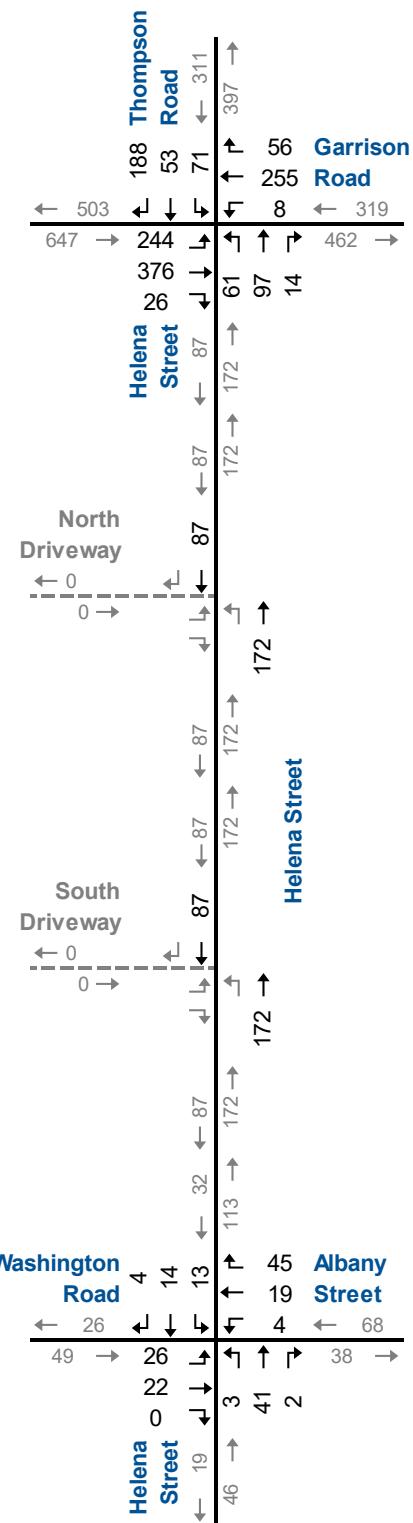
**Figure 4.1** illustrates the forecast background traffic volumes.

**Figure 4.2** illustrates the forecast total traffic volumes.

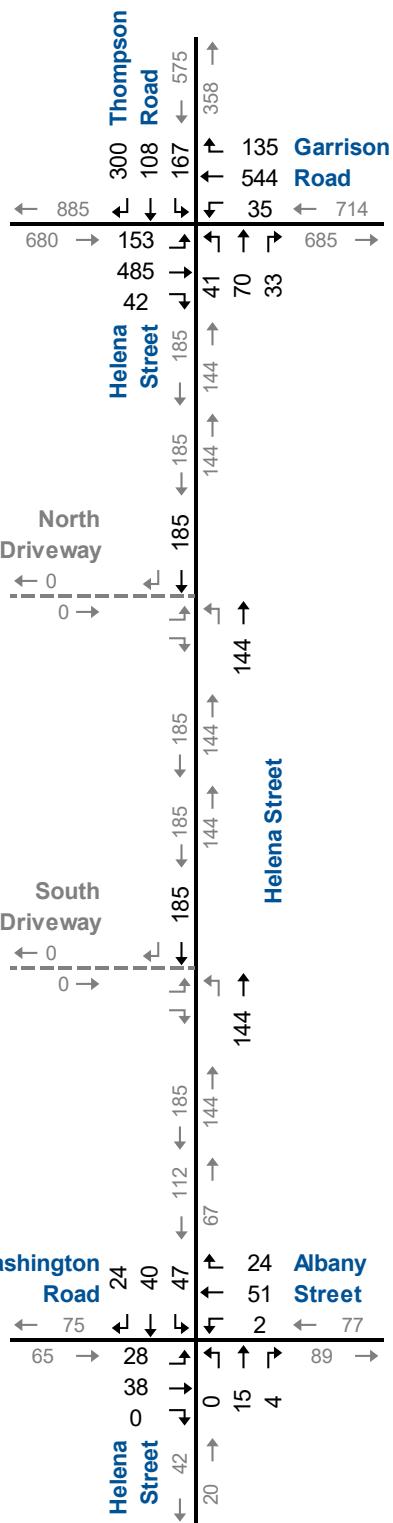




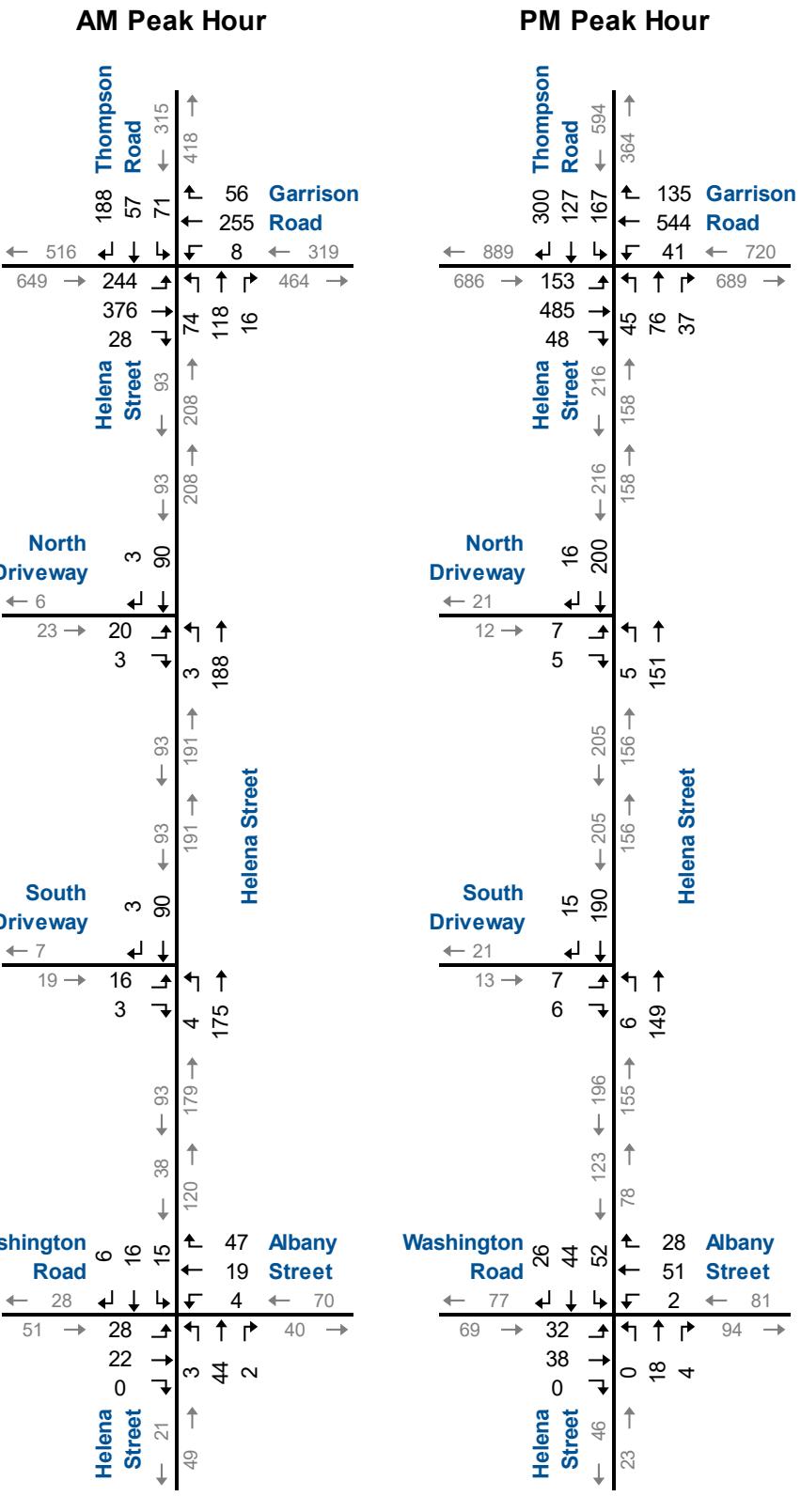
### AM Peak Hour



### PM Peak Hour



## Forecast Background Traffic Volumes



## 4.2 Background Traffic

The study area intersection operational analysis followed the same methodology used for existing conditions. No changes to the existing lane configurations or signal timings are assumed.

**Table 4.1** summarizes the level of service conditions. No critical movements are noted.

**Appendix D** contains the detailed Synchro 10 reports.



**TABLE 4.1: BACKGROUND TRAFFIC OPERATIONS**

Analysis Period	Intersection	Control Type	MOE	Direction / Movement / Approach																Overall	
				Eastbound				Westbound				Northbound				Southbound					
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach		
AM Peak Hour	Garrison Rd & Helena St / Thompson Rd	TCS	LOS	A	A	>	A 8	B	B	>	A 7	B	B	>	B 15	B	B	B	B 12 0.41		
			Delay	8	7	>		15	14	>		18	17	>		18	16	17	B 15		
PM Peak Hour	Helena St & Washington Rd / Albany St	AWSC	V/C	0.47	0.25	>	A 8	0.04	0.31	>	A 7	0.25	0.16	>	A 8	0.29	0.09	0.15	A 8		
			Q	37	24	>		6	34	>		24	20	>		24	21	2			
PM Peak Hour	Garrison Rd & Helena St / Thompson Rd	TCS	Ex	120	-	>	A 10	110	-	>	B 16	35	-	>	B 19	C	B	C	B 19 0.54		
			Avail.	83	-	>		104	-	>		12	-	>		24	19	20	B 19		
PM Peak Hour	Helena St & Washington Rd / Albany St	AWSC	LOS	<	A	>	A 8	<	A	>	A 8	<	8	>	A 8	<	A	>	A 8		
			Delay	<	8	>		<	7	>		<	0.06	>		<	8	>			
PM Peak Hour	Garrison Rd & Helena St / Thompson Rd	TCS	D.Util	<	0.06	>	A 10	<	0.08	>	B 16	<	0.13	>	B 19	0.56	0.14	0.24	B 19 0.54		
			Q	<	12	>		<	14	>		<	7	>		41	29	7			
PM Peak Hour	Helena St & Washington Rd / Albany St	AWSC	LOS	<	A	>	A 8	<	A	>	A 8	<	A	>	A 8	<	A	>	A 8		
			Delay	<	8	>		<	8	>		<	0.09	>		<	8	>			
PM Peak Hour	Garrison Rd & Helena St / Thompson Rd	TCS	V/C	0.44	0.34	>	A 10	0.12	0.55	>	B 16	0.10	0.19	>	B 19	0.24	0.19	0.20	B 19 0.54		
			Q	29	34	>		14	50	>		19	19	>		41	29	7			
PM Peak Hour	Helena St & Washington Rd / Albany St	AWSC	Ex	120	-	>	A 8	110	-	>	B 16	35	-	>	B 19	70	-	-	B 19 0.54		
			Avail.	91	-	>		96	-	>		16	-	>		29	-	-			

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

D.Util - Degree of Utilization

Q - 95th Percentile Queue Length

Ex. - Existing Available Storage

Avail. - Available Storage

&lt; - Shared Left-turn

&gt; - Shared Right-turn

TCS - Traffic Control Signal

AWSC - All-Way Stop Control

TWSC - Two-Way Stop Control



## 4.3 Total Traffic

The study area intersection operational analysis followed the same methodology used for existing conditions. No changes to the existing lane configurations or signal timings are assumed.

**Table 4.2** summarizes the level of service conditions. No critical movements are noted.

The site driveways are forecast to operate in the LOS A-B range with a v/c of less than 0.05 and 95<sup>th</sup> percentile queue lengths of less than 15 m.

**Appendix E** contains the detailed Synchro 10 reports.



**TABLE 4.2: TOTAL TRAFFIC OPERATIONS**

Analysis Period	Intersection	Control Type	MOE	Direction / Movement / Approach																Overall	
				Eastbound				Westbound				Northbound				Southbound					
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach		
AM Peak Hour	Garrison Rd & Helena St / Thompson Rd	TCS	LOS Delay V/C Q Ex Avail.	A 8 0.47 39 120 81	A 7 0.25 25 - -	> > > > >	A <b>7</b>	B 15 0.04 6 110 104	B 14 0.31 33 - -	> > > > >	B <b>14</b>	B 18 0.31 29 35 7	B 17 0.19 22 - -	> > > > >	B <b>17</b>	B 18 0.29 23 70 47	B 16 0.10 21 - -	B 17 0.15 2 -	B <b>12</b> <b>0.41</b>		
	Helena St & Washington Rd / Albany St	AWSC	LOS Delay D.Util Q	< < < <	A 8 0.06 11	> > > >	A <b>8</b>	< < < <	A 7 0.08 14	> > >	A <b>7</b>	< < < <	A 8 0.06 6	> > >	A <b>8</b>	< < < <	A 8 0.05 7	> > >	A <b>8</b>		
	North Driveway & Helena St	TWSC	LOS Delay V/C Q	B 10 0.04 13		> >	B <b>10</b>										A 0 0.06 -	> > >	A <b>0</b>		
	South Driveway & Helena St	TWSC	LOS Delay V/C Q	B 10 0.03 11		> >	B <b>10</b>										A 0 0.06 -	> > >	A <b>0</b>		
PM Peak Hour	Garrison Rd & Helena St / Thompson Rd	TCS	LOS Delay V/C Q Ex Avail.	A 10 0.44 33 120 87	A 10 0.34 37 - -	> > > > >	A <b>10</b>	B 14 0.14 15 110 95	B 16 0.55 50 - -	> > > > >	B <b>16</b>	B 20 0.15 19 35 17	B 19 0.11 20 - -	> > > > >	B <b>19</b>	C 24 0.56 42 70 29	B 19 0.16 35 - -	C 20 0.24 9 -	B <b>16</b> <b>0.54</b>		
	Helena St & Washington Rd / Albany St	AWSC	LOS Delay D.Util Q	< < < <	A 8 0.09 11	> > > >	A <b>8</b>	< < < <	A 8 0.10 12	> > >	A <b>8</b>	< < < <	A 8 0.03 5	> > >	A <b>8</b>	< < < <	A 8 0.16 13	> > >	A <b>8</b>		
	North Driveway & Helena St	TWSC	LOS Delay V/C Q	A 10 0.01 6		> >	A <b>10</b>										A 0 0.00 3	> > >	A <b>0</b>		
	South Driveway & Helena St	TWSC	LOS Delay V/C Q	B 10 0.02 10		> >	B <b>10</b>										A 0 0.01 3	> > >	A <b>0</b>		

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

D.Util - Degree of Utilization

Q - 95th Percentile Queue Length

Ex. - Existing Available Storage

Avail. - Available Storage

&lt; - Shared Left-turn

&gt; - Shared Right-turn

TCS - Traffic Control Signal

AWSC - All-Way Stop Control

TWSC - Two-Way Stop Control



## 5 Remedial Measures

### 5.1 Auxiliary Left-Turn Lanes

The need for left-turn lanes on Helena Street at the site driveways were evaluated using the Ministry of Transportation's (MTO) Design Supplement for the Transportation Association of Canada (TAC) Geometric Design Guide for Canadian Roads<sup>9</sup>. The warrant nomographs determine if a left-turn lane is warranted based on the following criteria:

- ▶ Design speed of the road (20 km/h over the posted speed limit);
- ▶ Advancing volume;
- ▶ Opposing volume; and
- ▶ Percentage of advancing vehicles performing a left-turn movement.

**Appendix F** contains the left-turn lane warrant nomographs. The nomographs indicate that left-turn lanes are not warranted on Helena Street at the site driveways.

---

<sup>9</sup> MTO Design Supplement for TAC Geometric Design Guide for Canadian Roads, June 2017 Appendix 9 for Chapter 9 Intersections



# 6 Conclusions and Recommendations

## 6.1 Conclusions

Based on the investigations carried out, it is concluded that:

- ▶ **Base Year Traffic Conditions:** The study area intersections are operating at acceptable levels of service and all movements are well within capacity during the weekday AM and PM peak hours. No critical movements are noted.
- ▶ **Site Trip Generation:** The proposed development is estimated to generate approximately 55 AM peak hour vehicle trips and 67 PM peak hour vehicle trips.
- ▶ **Background Traffic Conditions:** The study area intersections are forecast to operate at acceptable levels of service and all movements are well within capacity during the weekday AM and PM peak hours. No critical movements are noted.
- ▶ **Total Traffic Conditions:** The study area intersections are forecast to operate at acceptable levels of service and all movements are well within capacity during the weekday AM and PM peak hours. No critical movements are noted.

The site driveways are forecast to operate in the LOS A-B range with a v/c of less than 0.05 and 95<sup>th</sup> percentile queue lengths of less than 15 m.

- ▶ **Remedial Measures:** No improvements to the existing and proposed form of traffic control are required at the study area intersections.

Northbound left-turn lanes on Helena Street at the site driveways are not warranted based on forecast traffic volumes.

## 6.2 Recommendations

Based on the findings of this study, it is recommended that the site driveway approaches to Helena Street operate as unsignalized intersections.



# Appendix A

## Pre-Study Consultation



## **Stefan Hajgato**

---

**From:** Jeremy Korevaar <JKorevaar@forterie.ca>  
**Sent:** July 27, 2021 9:47 AM  
**To:** Stefan Hajgato  
**Cc:** Scott Catton  
**Subject:** RE: (210371: 613 Helena St) TIS Terms of Reference

Good Morning Stefan,

If the Region is accepting of factored traffic counts, that is fine with me.

Regards,

**Jeremy Korevaar, C.E.T.**  
Coordinator, Development Approvals

**Town of Fort Erie**

1 Municipal Centre Drive  
Fort Erie, Ontario  
Canada  
L2A 2S6

TEL: 1-905-871-1600 ext.2505  
FAX: 1-905-871-6411

---

From: "Stefan Hajgato" <shajgato@ptsl.com>  
To: "Jeremy Korevaar (JKorevaar@forterie.ca)" <JKorevaar@forterie.ca>  
Cc: "Scott Catton" <scatton@ptsl.com>  
Date: 2021-07-26 10:33 AM  
Subject: RE: (210371: 613 Helena St) TIS Terms of Reference

---

Hi Jeremy,

As mentioned in Susan's email, the Region does not require additional turning movement counts to be collected. Do you have any concerns with using historical data factored to 2021?

Thanks,

**Stefan Hajgato, P.Eng.**  
*Transportation Engineer*  
(He/Him)



**Paradigm Transportation Solutions Limited**  
p: 519.896.3163 x209

**From:**Dunsmore, Susan <Susan.Dunsmore@niagararegion.ca>  
**Sent:** July 26, 2021 8:55 AM  
**To:** Stefan Hajgato <shajgato@ptsl.com>  
**Cc:** Jeremy Korevaar (JKorevaar@forterie.ca) <JKorevaar@forterie.ca>; Ramundo, Matteo <Matteo.Ramundo@niagararegion.ca>; Scott Catton <scatton@ptsl.com>  
**Subject:** RE: (210371: 613 Helena St) TIS Terms of Reference

Hello Stefan,

Regional transportation staff have reviewed your TIS terms of reference and our comments are noted below in green.

If there are any improvements required a functional design should be included in the TIS.

If you require any additional information please contact me at your convenience.

Thank you

**Susan M. Dunsmore, P. Eng.**  
Manager, Development Engineering  
Planning and Development Services

Phone:(905) 980-6000 or 1-800-263-7215 ext 3661  
Address: 1815 Sir Isaac Brock Way, Thorold ON, L2V4T7



**From:**Stefan Hajgato <[shajgato@ptsl.com](mailto:shajgato@ptsl.com)>  
**Sent:** Monday, July 12, 2021 3:33 PM  
**To:** Dunsmore, Susan <[Susan.Dunsmore@niagararegion.ca](mailto:Susan.Dunsmore@niagararegion.ca)>; [JKorevaar@forterie.ca](mailto:JKorevaar@forterie.ca)  
**Cc:** Scott Catton <[scatton@ptsl.com](mailto:scatton@ptsl.com)>  
**Subject:** (210371: 613 Helena St) TIS Terms of Reference

**CAUTION:**This email originated from outside of the Niagara Region email system. Use caution when clicking links or opening attachments unless you recognize the sender and know the content is safe.

Hi Susan and Jeremy,

Paradigm has been retained to prepare a Transportation Impact Study (TIS) for a proposed 116-unit townhouse development at 613 Helena Street in the Town of Fort Erie. Two driveways are proposed to Helena Street approximately 475 and 560 metres north of Washington Road / Albany Street.

We are proposing the following scope:

**Study Area Intersections:**

- Helena Street / Thompson Road at Garrison Road (signalized); Accepted
- Helena Street at Washington Road / Albany Street (unsignalized); and Accepted

- Proposed site driveways (up to 2) to Helena Street. Accepted

**Development** (site plan attached):

- 116 townhouse units (bungalow and standard units).
- Build-out - Year 2024.

**Horizon Year:**

- Existing (Year 2021)
- Year 2026 (5-years from TIS commission).Accepted.

**Growth Rate:** 2.0% per annum.Accepted.

**Existing Traffic Volumes:** Please see discussion below.

**Background Developments:** Please identify.

**Analysis Periods:** Weekday AM & PM peak hours.Accepted.

**Trip Generation:** ITE Trip Generation Manual 10e.Accepted.

**Trip Distribution:** Local travel patterns. Accepted.

**Planned Roadway Improvements:** Please identify.

**Capacity Analysis Software:** Synchro. Paradigm to confirm the version of Synchro they are using.

**Additional Studies:** Paradigm to update their classification/speed study on Helena Street adjacent to the site that was requested and prepared for the 2018 TIS for the same previous reason. The purpose of this study is to address concerns that vehicles may be speeding on this road which could cause unsafe conditions for vehicles entering and exiting the subject site.

**Existing Traffic Volumes**

We can factor historical turning movements counts to 2021 conditions. Paradigm currently has 2016 counts at the Helena Street / Thompson Road and Garrison Road intersection and 2018 counts at the Helena Street and Washington Road / Albany Street intersection. Please advise if the Town or Region is able to provide more recent data.

As per the Region current TIS guidelines, traffic counts must be within at least 3 years of the study (ideally undertaken prior to the start of the pandemic in March 2020). 2018 counts for Helena Street and Washington Road/Albany Street intersection are accepted. The Region has 2019 counts for Helena Street/Thompson Road and Garrison Road intersection that can be purchased by Paradigm. In order to reflect todays counts we would ask that the older counts be increased by 2% each year to reach the anticipated 2021 counts. Regional counts can be obtained through our online system at the following link:  
<https://www.niagararegion.ca/living/roads/permits/traffic-data-requests.aspx>

Alternatively, we can collect current turning movement counts at the study intersections. However, traffic patterns may be altered due to potential / recently lifted travel restrictions at the time of data collection. Please advise if you would like us to go this route and if we should apply a factor to the counts to normalize volumes. New traffic counts are not required as per above.

Feel free to reach out if you have any questions or comments.

Thanks,

**Stefan Hajgato, P.Eng.**

*Transportation Engineer*

*(He/Him)*



**Paradigm Transportation Solutions Limited**

150 Pinebush Road, Unit 5A, Cambridge ON N1R 8J8

p: 519.896.3163 x209

e: [shajgato@ptsl.com](mailto:shajgato@ptsl.com)

w: [www.ptsl.com](http://www.ptsl.com)

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Thanks for connecting.

For up-to-date information about the Town of Fort Erie's response to the COVID-19 virus, please visit  
<https://www.forterie.ca/pages/Covid19News>

We also strongly encourage you to stay-up-to-date with the latest advice from Niagara Region Public Health at  
<https://www.niagararegion.ca/health/covid-19/default.aspx>

All critical services, including water and wastewater and road operations as well as fire services will continue to operate to support our community. For after-hours services, including road or wastewater operations, please call 905-871-1600.

For emergency assistance from fire services please call 911.

As many Town staff are focusing on emergency management at this time, we thank you in advance for your patience.

## Appendix B

### Existing Data



## Helena St @ Albany St

### Morning Peak Diagram

#### Specified Period

**From:** 6:00:00

**To:** 9:00:00

#### One Hour Peak

**From:** 8:00:00

**To:** 9:00:00

**Municipality:** Fort Erie

**Site #:** 0000000001

**Intersection:** Helena St & Albany St

**TFR File #:** 1

**Count date:** 16-Jan-2018

#### Weather conditions:

Clear/Wet

#### Person(s) who counted:

Cam

#### \*\* Non-Signalized Intersection \*\*

**Major Road:** Helena St runs N/S

North Leg Total: 131

North Entering: 29

North Peds: 0

Peds Cross: ☒

Heavys	0	0	1	1
Trucks	1	0	0	1
Cars	3	13	11	27
Totals	4	13	12	

East Leg Total: 96

East Entering: 62

East Peds: 0

Peds Cross: ☐

Heavys	3	1	20	24
Trucks				
Cars				
Totals				

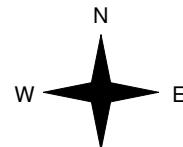


Helena St

Cars	39	0	2	41
Trucks	16	0	1	17
Heavy	3	0	1	4
Totals	58	0	4	

Heavys	0	0	24	24
Trucks	1	0	19	20
Cars	0	0	0	0
Totals	1	0	43	

Washington Rd



Albany St

Cars	32	0	2	34
Trucks				
Heavy				
Totals				

Peds Cross:	☒
West Peds:	0
West Entering:	44
West Leg Total:	68

Cars	16			
Trucks	0			
Heavy	1			
Totals	17			

Cars	1	37	2	40
Trucks	0	0	0	0
Heavy	2	0	0	2
Totals	3	37	2	

Peds Cross:	☒
South Peds:	0
South Entering:	42
South Leg Total:	59

### Comments

# Helena St @ Albany St

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00

**To:** 13:00:00

### One Hour Peak

**From:** 12:00:00

**To:** 13:00:00

**Municipality:** Fort Erie

**Site #:** 0000000001

**Intersection:** Helena St & Albany St

**TFR File #:** 1

**Count date:** 16-Jan-2018

### Weather conditions:

Clear/Wet

### Person(s) who counted:

Cam

### \*\* Non-Signalized Intersection \*\*

**Major Road:** Helena St runs N/S

North Leg Total: 140

North Entering: 57

North Peds: 0

Peds Cross: ☒

Heavys	0	0	0	0
Trucks	1	1	1	3
Cars	16	28	10	54
Totals	17	29	11	

East Leg Total: 89

East Entering: 54

East Peds: 0

Peds Cross: ☒

Heavys Trucks Cars Totals

1	1	42	44
---	---	----	----



Helena St

Washington Rd

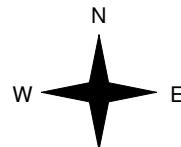
Heavys Trucks Cars Totals

1	1	28	30
---	---	----	----

0	0	24	24
---	---	----	----

0	0	0	0
---	---	---	---

1	1	52	
---	---	----	--



Cars	Trucks	Heavys	Totals
22	1	0	23
25	0	1	26
4	0	1	5
51	1	2	

Albany St



Cars	Trucks	Heavys	Totals
34	1	0	35

Peds Cross: ☒

West Peds: 0

West Entering: 54

West Leg Total: 98

Cars 32

Trucks 1

Heavys 1

Totals 34

Cars	29	0	30
------	----	---	----

Trucks	0	1	1
--------	---	---	---

Heavys	0	0	0
--------	---	---	---

Totals	1	30	0
--------	---	----	---

Peds Cross: ☐

South Peds: 3

South Entering: 31

South Leg Total: 65

## Comments

## Helena St @ Albany St

### Afternoon Peak Diagram

#### Specified Period

**From:** 15:00:00

**To:** 18:00:00

#### One Hour Peak

**From:** 16:30:00

**To:** 17:30:00

**Municipality:** Fort Erie

**Site #:** 0000000001

**Intersection:** Helena St & Albany St

**TFR File #:** 1

**Count date:** 16-Jan-2018

#### Weather conditions:

Clear/Wet

#### Person(s) who counted:

Cam

#### \*\* Non-Signalized Intersection \*\*

**Major Road:** Helena St runs N/S

North Leg Total: 162

North Entering: 101

North Peds:

Peds Cross: ☒

Heavys	0	1	0	1
Trucks	0	0	0	0
Cars	22	35	43	100
Totals	22	36	43	

Heavys 1

Trucks 0

Cars 60

Totals 61

East Leg Total: 151

East Entering: 70

East Peds:

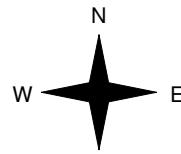
Peds Cross: ☒

Heavys Trucks Cars Totals  
1 0 67 68



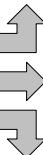
Helena St

Washington Rd



Cars	Trucks	Heavys	Totals
22	0	0	22
45	0	1	46
1	0	1	2
68	0	2	

Heavys Trucks Cars Totals  
0 0 25 25  
0 1 33 34  
0 0 0 0  
0 1 58



Albany St



Cars	Trucks	Heavys	Totals
79	1	1	81

Peds Cross: ☒  
West Peds: 0  
West Entering: 59  
West Leg Total: 127

Cars 36  
Trucks 0  
Heavys 2  
Totals 38

Cars	0	13	3	16
Trucks	0	0	0	0
Heavys	0	1	1	2
Totals	0	14	4	

Peds Cross: ☐  
South Peds: 0  
South Entering: 18  
South Leg Total: 56

### Comments

# **Helena St @ Albany St**

## **Total Count Diagram**

**Municipality:** Fort Erie

**Site #:** 0000000001

**Intersection:** Helena St & Albany St

**TFR File #:** 1

**Count date:** 16-Jan-2018

**Weather conditions:**

Clear/Wet

**Person(s) who counted:**

Cam

**\*\* Non-Signalized Intersection \*\***

**Major Road:** Helena St runs N/S

North Leg Total: 995

North Entering: 452

North Peds:

Peds Cross: ☒

Heavys	0	1	2	3
Trucks	4	1	1	6
Cars	119	194	130	443
Totals	123	196	133	

Heavys 15

Trucks 4

Cars 524

Totals 543

East Leg Total: 697

East Entering: 390

East Peds:

Peds Cross: ☒

Heavys Trucks Cars Totals

8 7 300 315



Helena St

Washington Rd

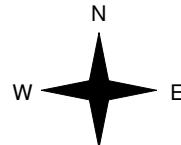
Heavys Trucks Cars Totals

3 1 163 167

3 2 152 157

0 0 1 1

6 3 316



Cars	Trucks	Heavys	Totals
171	1	8	180
178	3	6	187
18	0	5	23
367	4	19	

Albany St



Peds Cross: ☒

West Peds: 2

West Entering: 325

West Leg Total: 640

Cars 213

Trucks 1

Heavys 6

Totals 220

Cars 3

Trucks 0

Heavys 2

Totals 5

190

2

4

196

13

0

4

17

206

2

10

Peds Cross: ☐

South Peds: 6

South Entering: 218

South Leg Total: 438

### **Comments**

**Location.....** Garrison Road @ Helena Street/Thompson Road

**GeoID.....** 01725

**Municipality.** FORT ERIE

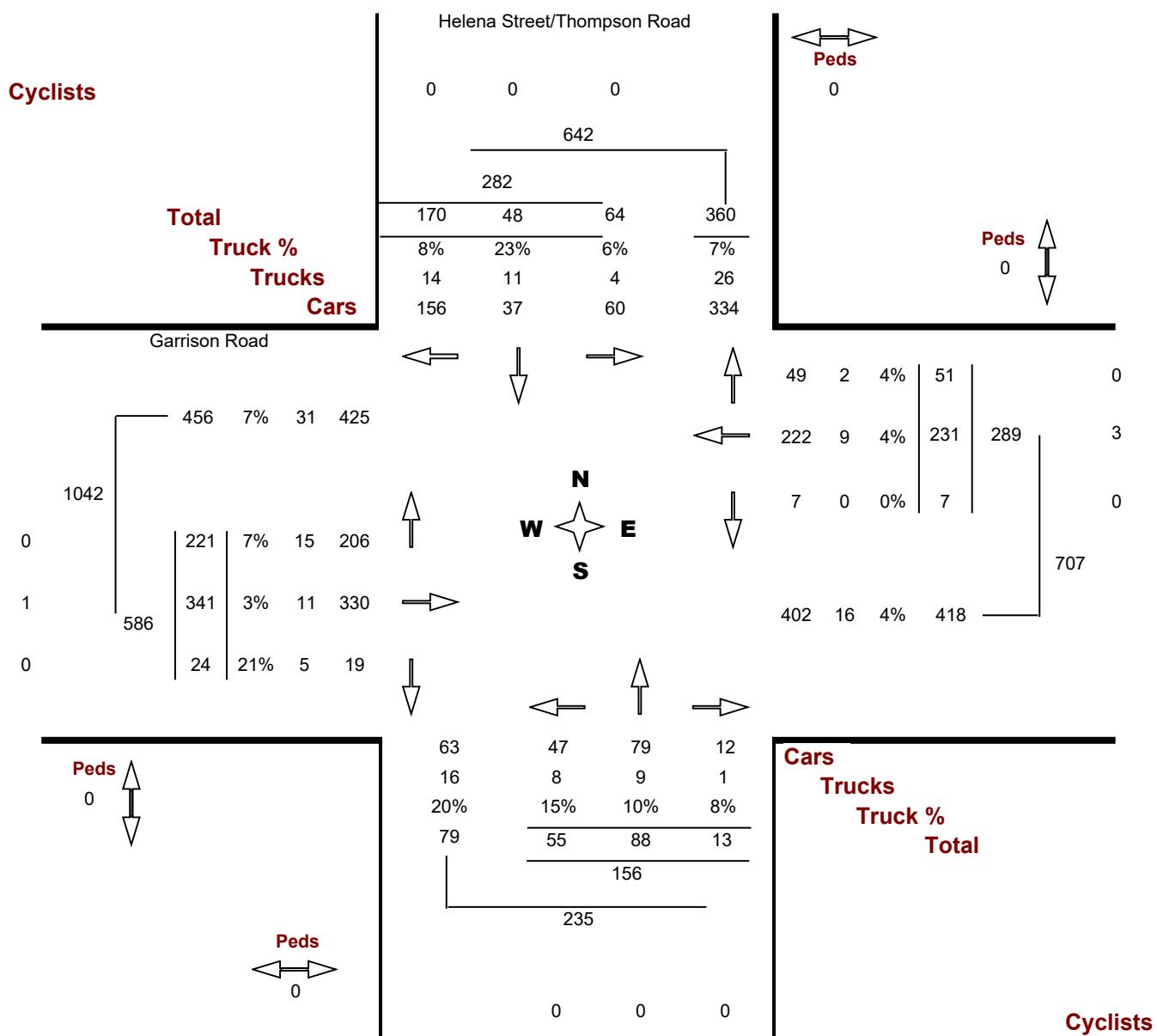
**Count Date.** Thursday, 23 May, 2019

**Traffic Cont.**

**Count Time.** 07:00 AM — 09:00 AM

**Major Dir.....** East west

**Peak Hour..** 08:00 AM — 09:00 AM



**Location.....** Garrison Road @ Helena Street/Thompson Road

**GeOID.....** 01725

**Municipality.** FORT ERIE

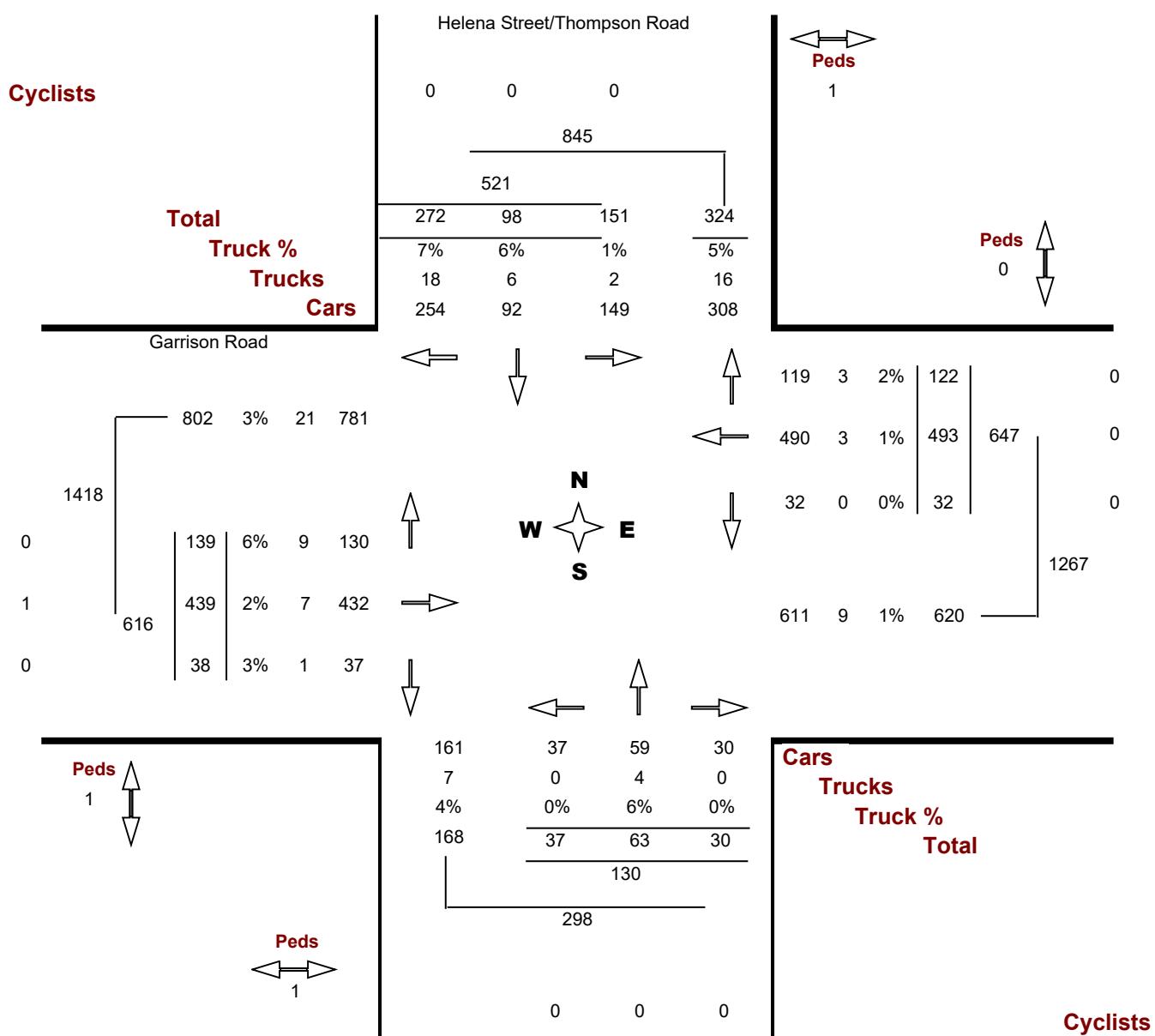
**Count Date.** Thursday, 23 May, 2019

**Traffic Cont.**

**Count Time.** 03:00 PM — 06:00 PM

**Major Dir.....** East west

**Peak Hour..** 03:30 PM — 04:30 PM



**Location.....** Garrison Road @ Helena Street/Thompson Road

**GeoID.....** 01725

**Municipality.** FORT ERIE

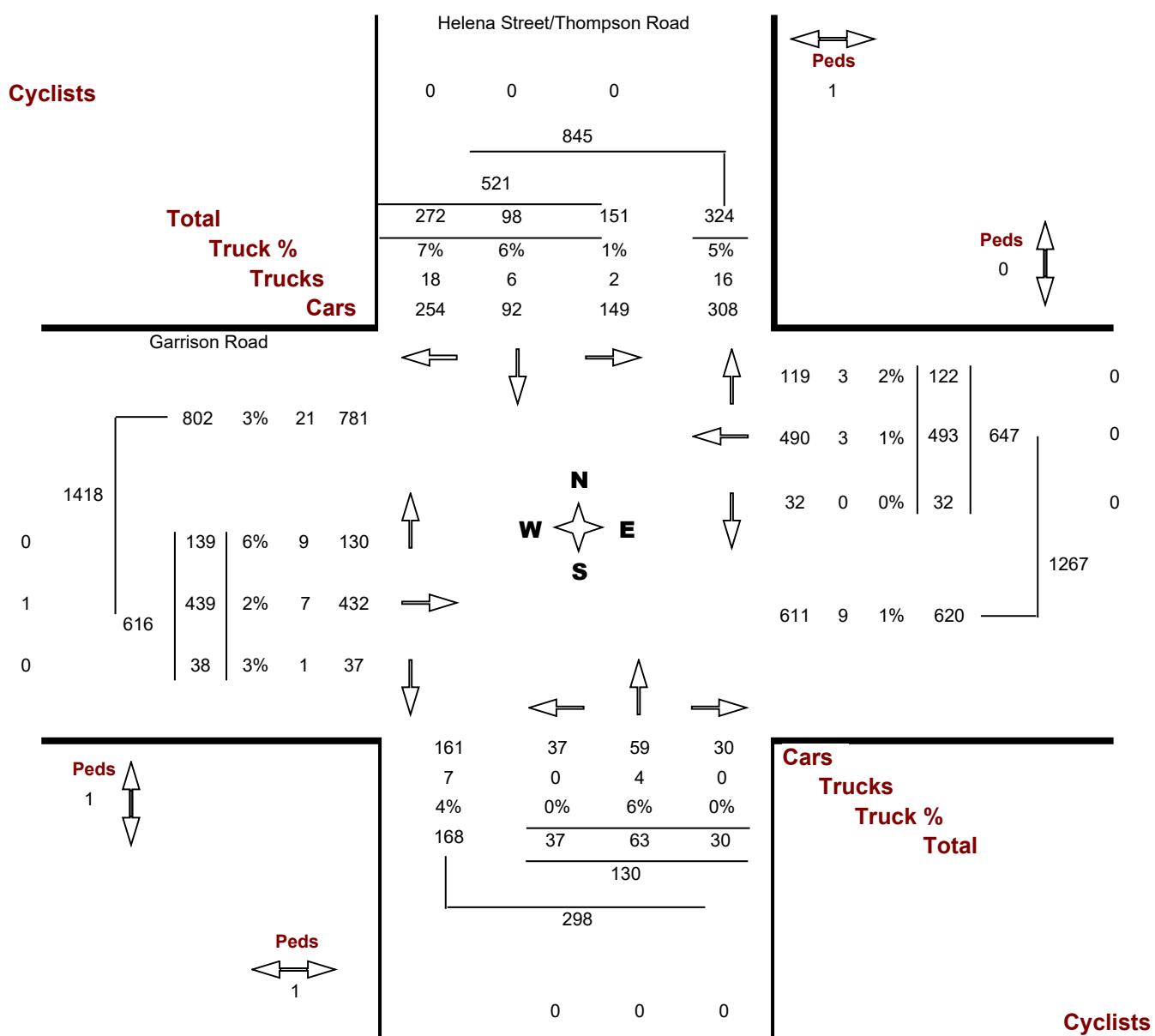
**Count Date.** Thursday, 23 May, 2019

**Traffic Cont.**

**Count Time.** 07:00 AM — 06:00 PM

**Major Dir.....** East west

**Peak Hour..** 03:30 PM — 04:30 PM



## Turning Movement Count - Details Report (15 min)

**Location.....** Garrison Road @ Helena Street/Thompson Road

**Municipality.....** FORT ERIE

**Count Date.....** Thursday, May 23, 2019

Helena Street/Thompson Road

Garrison Road

North Approach					South Approach					East Approach					West Approach					
Time Period	LT	TH	RT	U-Turn	TOT	LT	TH	RT	U-Turn	TOT	LT	TH	RT	U-Turn	TOT	LT	TH	RT	U-Turn	TOT

07:00	07:15	12	10	22	0	44	3	15	2	0	20	2	16	5	0	23	21	44	2	0	67
07:15	07:30	5	10	21	0	36	4	12	2	0	18	1	27	8	0	36	42	43	5	0	90
07:30	07:45	5	5	27	0	37	6	24	3	0	33	2	36	8	0	46	36	53	4	0	93
07:45	08:00	13	9	46	0	68	12	20	4	0	36	2	62	7	0	71	44	80	3	0	127
<b>Hourly Total</b>		35	34	116	0	185	25	71	11	0	107	7	141	28	0	176	143	220	14	0	377
08:00	08:15	12	11	41	0	64	13	16	1	0	30	2	64	9	0	75	42	67	3	0	112
08:15	08:30	8	10	43	0	61	12	22	4	0	38	3	61	13	0	77	52	85	3	0	140
08:30	08:45	23	17	46	0	86	18	26	3	0	47	1	48	14	0	63	55	80	7	0	142
08:45	09:00	21	10	40	0	71	12	24	5	0	41	1	58	15	0	74	72	109	11	0	192
<b>Hourly Total</b>		64	48	170	0	282	55	88	13	0	156	7	231	51	0	289	221	341	24	0	586
11:00	11:15	27	18	30	0	75	4	15	5	0	24	3	105	14	0	122	28	94	3	0	125
11:15	11:30	32	15	37	0	84	6	11	4	0	21	5	93	31	0	129	35	86	4	0	125
11:30	11:45	32	13	39	0	84	7	17	3	0	27	4	80	24	0	108	36	78	6	0	120
11:45	12:00	32	22	39	0	93	12	15	6	0	33	3	97	30	0	130	26	125	2	0	153
<b>Hourly Total</b>		123	68	145	0	336	29	58	18	0	105	15	375	99	0	489	125	383	15	0	523
12:00	12:15	38	14	49	0	101	9	13	5	0	27	3	128	27	0	158	33	102	13	0	148
12:15	12:30	39	18	25	0	82	12	18	3	0	33	3	112	19	0	134	35	112	13	0	160
12:30	12:45	33	16	41	0	90	4	16	8	0	28	2	104	35	0	141	30	106	5	0	141
12:45	13:00	44	16	35	0	95	5	13	4	0	22	7	90	22	0	119	47	110	8	0	165
<b>Hourly Total</b>		154	64	150	0	368	30	60	20	0	110	15	434	103	0	552	145	430	39	0	614
13:00	13:15	42	6	35	0	83	5	16	9	0	30	4	94	23	0	121	33	110	5	0	148
13:15	13:30	36	9	41	0	86	7	19	6	0	32	7	81	32	0	120	36	101	2	0	139
13:30	13:45	37	15	28	0	80	8	7	9	0	24	5	98	19	0	122	37	82	11	0	130
13:45	14:00	35	15	44	0	94	3	12	8	0	23	2	98	27	0	127	47	95	8	0	150
<b>Hourly Total</b>		150	45	148	0	343	23	54	32	0	109	18	371	101	0	490	153	388	26	0	567
15:00	15:15	31	18	65	0	114	10	16	3	0	29	3	98	25	0	126	41	105	3	0	149
15:15	15:30	47	22	35	0	104	9	28	7	0	44	9	120	40	0	169	40	100	14	0	154
15:30	15:45	36	19	69	0	124	11	20	9	0	40	10	109	34	0	153	34	96	15	0	145
15:45	16:00	47	34	73	0	154	10	9	6	0	25	5	102	30	0	137	39	133	8	0	180
<b>Hourly Total</b>		161	93	242	0	496	40	73	25	0	138	27	429	129	0	585	154	434	40	0	628
16:00	16:15	41	19	63	0	123	8	21	8	0	37	3	135	30	0	168	28	89	8	0	125

## Helena Street/Thompson Road

## Garrison Road

Time Period	North Approach				South Approach				East Approach				West Approach								
	LT	TH	RT	U-Turn	TOT	LT	TH	RT	U-Turn	TOT	LT	TH	RT	U-Turn	TOT	LT	TH	RT	U-Turn	TOT	
16:15	16:30	27	26	67	0	120	8	13	7	0	28	14	147	28	0	189	38	121	7	0	166
16:30	16:45	33	35	63	0	131	10	16	8	0	34	5	106	26	0	137	29	96	8	0	133
16:45	17:00	21	23	56	0	100	4	13	7	0	24	6	117	20	0	143	39	103	6	0	148
Hourly Total		122	103	249	0	474	30	63	30	0	123	28	505	104	0	637	134	409	29	0	572
17:00	17:15	43	28	71	0	142	8	19	3	0	30	5	109	27	0	141	38	110	9	0	157
17:15	17:30	23	21	50	0	94	3	13	8	0	24	4	118	26	0	148	38	97	5	0	140
17:30	17:45	34	20	74	0	128	3	8	4	0	15	7	98	13	0	118	33	87	5	0	125
17:45	18:00	30	22	50	0	102	7	12	4	0	23	4	95	21	0	120	34	92	6	0	132
Hourly Total		130	91	245	0	466	21	52	19	0	92	20	420	87	0	527	143	386	25	0	554
Grand Total		939	546	1465	0	2950	253	519	168	0	940	137	2906	702	0	3745	1218	2991	212	0	4421
Truck %		2%	9%	6%	0%	5%	8%	8%	5%	0%	7%	1%	2%	4%	0%	2%	6%	2%	8%	0%	4%

<b>Signal Code: 003122</b>						
<b>Intersection: RR3(Garrison) &amp; RR122(Thompson Rd.)</b>						
<b>Municipality:</b> fort Erie						
<b>Owner:</b> Region						
<b>Last Modified:</b> 1/11/2018 8:51:15 AM						
<b>Timing Parameters</b>	<b>EBD &amp; WBD ADVANCE GARRISON RD.</b>	<b>EBD &amp; WBD THRU GARRISON RD.</b>	<b>NBD &amp; SBD THRU THOMPSON RD./HELENA ST.</b>	n/a	n/a	n/a
<b>Min Green</b>	<b>6</b>	<b>10</b>	<b>10</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>Walk</b>	<b>0</b>	<b>12</b>	<b>12</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>Ped Clearance</b>	<b>0</b>	<b>20</b>	<b>20</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>Vehicle Ext.</b>	<b>2.5</b>	<b>5</b>	<b>3</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>Max Green</b>	<b>25</b>	<b>35</b>	<b>20</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>Yellow</b>	<b>3</b>	<b>5</b>	<b>4.1</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>All Red</b>	<b>0</b>	<b>2</b>	<b>2</b>	<b>0</b>	<b>0</b>	<b>0</b>

		Offset
<b>Minimum Cycle</b>	<b>33.1</b>	<b>0</b>
<b>Pedestrian Cycle</b>	<b>77.1</b>	
<b>Maximum Cycle</b>	<b>96.1</b>	<b>0</b>
<b>Operation</b>	<b>FA</b>	

**Installed On:****3/6/2004****Count Date:****10/13/2016**

FA = Fully Actuated

SA = Semi Actuated

FT = Fixed Time

  
**\*Note: you need to change the paper orientation from Portrait to Landscape**

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**MH Corbin Traffic Analyzer Study**  
**Computer Generated Summary Report**  
**City: Fort Erie**  
**Street: 613 Helena St - NB**  
**Location: 1**

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A study of vehicle traffic was conducted with the device having serial number 405258. The study was done in the NB lane at 613 Helena St - NB in Fort Erie, ON in county. The study began on 2021-07-28 at 12:00 AM and concluded on 2021-07-29 at 12:00 AM, lasting a total of 24.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 1,140 vehicles passed through the location with a peak volume of 31 on 2021-07-28 at [02:30 PM-02:45 PM] and a minimum volume of 0 on 2021-07-28 at [10:30 PM-10:45 PM]. The AADT count for this study was 1,140.

### **SPEED**

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 70 - 80 KM/H range or lower. The average speed for all classified vehicles was 73 KM/H with 59.07% vehicles exceeding the posted speed of 70 KM/H. 26.23% percent of the total vehicles were traveling in excess of 89 KM/H. The mode speed for this traffic study was 70KM/H and the 85th percentile was 86.65 KM/H.

< to 9	10 to 19	20 to 29	30 to 39	40 to 49	50 to 59	60 to 69	70 to 79	80 to 89	90 to 99	100 to 109	110 to 119	120 to 129	130 to 139	140 to >
0	3	2	3	18	118	321	373	191	76	15	9	7	0	0

CHART 1

### **CLASSIFICATION**

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin. Most of the vehicles classified during the study were Passenger Vehicles. The number of Passenger Vehicles in the study was 1094 which represents 96 percent of the total classified vehicles. The number of Small Trucks in the study was 7 which represents 1 percent of the total classified vehicles. The number of Trucks/Buses in the study was 27 which represents 2 percent of the total classified vehicles. The number of Tractor Trailers in the study was 8 which represents 1 percent of the total classified vehicles.

< to 4.9	5.0 to 8.4	8.5 to 9.9	10.0 to 12.9	13.0 to 15.9	16.0 to 18.9	19.0 to 22.4	22.5 to >							
485	609	7	27	7	1	0	0							

CHART 2

### **HEADWAY**

During the peak traffic period, on 2021-07-28 at [02:30 PM-02:45 PM] the average headway between vehicles was 28.125 seconds. During the slowest traffic period, on 2021-07-28 at [10:30 PM-10:45 PM] the average headway between vehicles was 900 seconds.

### **WEATHER**

The roadway surface temperature over the period of the study varied between 22.00 and 44.00 degrees C.

**MH Corbin Traffic Analyzer Study**  
**Computer Generated Summary Report**  
**City: Fort Erie**  
**Street: 613 Helena St - SB**  
**Location: 1**

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A study of vehicle traffic was conducted with the device having serial number 400181. The study was done in the SB lane at 613 Helena St - SB in Fort Erie, ON in county. The study began on 2021-07-28 at 12:00 AM and concluded on 2021-07-29 at 12:00 AM, lasting a total of 24.00 hours. Traffic statistics were recorded in 15 minute time periods. The total recorded volume showed 1,284 vehicles passed through the location with a peak volume of 46 on 2021-07-28 at [05:00 PM-05:15 PM] and a minimum volume of 0 on 2021-07-28 at [01:30 AM-01:45 AM]. The AADT count for this study was 1,284.

### **SPEED**

Chart 1 lists the values of the speed bins and the total traffic volume for each bin. At least half the vehicles were traveling in the 70 - 80 KM/H range or lower. The average speed for all classified vehicles was 74 KM/H with 62.67% vehicles exceeding the posted speed of 70 KM/H. 24.16% percent of the total vehicles were traveling in excess of 89 KM/H. The mode speed for this traffic study was 70KM/H and the 85th percentile was 86.00 KM/H.

< to 9	10 to 19	20 to 29	30 to 39	40 to 49	50 to 59	60 to 69	70 to 79	80 to 89	90 to 99	100 to 109	110 to 119	120 to 129	130 to 139	140 to >
0	2	3	2	12	105	355	494	195	82	27	3	3	0	0

CHART 1

### **CLASSIFICATION**

Chart 2 lists the values of the classification bins and the total traffic volume accumulated for each bin. Most of the vehicles classified during the study were Passenger Vehicles. The number of Passenger Vehicles in the study was 1235 which represents 96 percent of the total classified vehicles. The number of Small Trucks in the study was 14 which represents 1 percent of the total classified vehicles. The number of Trucks/Buses in the study was 26 which represents 2 percent of the total classified vehicles. The number of Tractor Trailers in the study was 8 which represents 1 percent of the total classified vehicles.

< to 4.9	5.0 to 8.4	8.5 to 9.9	10.0 to 12.9	13.0 to 15.9	16.0 to 18.9	19.0 to 22.4	22.5 to >							
713	522	14	26	5	1	2	0							

CHART 2

### **HEADWAY**

During the peak traffic period, on 2021-07-28 at [05:00 PM-05:15 PM] the average headway between vehicles was 19.149 seconds. During the slowest traffic period, on 2021-07-28 at [01:30 AM-01:45 AM] the average headway between vehicles was 900 seconds.

### **WEATHER**

The roadway surface temperature over the period of the study varied between 22.00 and 45.00 degrees C.

### Class/Volume Report Graph

HI-Star ID: 62F0A  
 Street: 613 Helena St - NB  
 State: ON  
 City: Fort Erie  
 Area:

Begin: 2021-07-28 12:00 AM  
 Lane: NB  
 Oper: MD  
 Posted: 70  
 AADT Factor: 1  
 End: 2021-07-29 12:00 AM  
 Hours: 24:00  
 Period: 15  
 Raw Count: 1140  
 AADT Count: 1140

NC300 - Metres	0.0 to 4.9	5.0 to 8.4	8.5 to 9.9	10.0 to 12.9	13.0 to 15.9	16.0 to 18.9	19.0 to 22.4	22.5 >	Total
2021-07-28 [12:00 AM-12:15 AM]	1	1	0	0	0	0	0	0	1
2021-07-28 [12:15 AM-12:30 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [12:30 AM-12:45 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [12:45 AM-01:00 AM]	0	0	0	0	0	0	0	0	0
	1	1	0	0	0	0	0	0	1
2021-07-28 [01:00 AM-01:15 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [01:15 AM-01:30 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [01:30 AM-01:45 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [01:45 AM-02:00 AM]	0	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0	0
2021-07-28 [02:00 AM-02:15 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [02:15 AM-02:30 AM]	1	0	1	0	0	0	0	0	1
2021-07-28 [02:30 AM-02:45 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [02:45 AM-03:00 AM]	0	0	0	0	0	0	0	0	0
	1	0	1	0	0	0	0	0	1
2021-07-28 [03:00 AM-03:15 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [03:15 AM-03:30 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [03:30 AM-03:45 AM]	1	1	0	0	0	0	0	0	1
2021-07-28 [03:45 AM-04:00 AM]	0	0	0	0	0	0	0	0	0
	1	1	0	0	0	0	0	0	1
2021-07-28 [04:00 AM-04:15 AM]	1	1	0	0	0	0	0	0	1
2021-07-28 [04:15 AM-04:30 AM]	2	2	0	0	0	0	0	0	2
2021-07-28 [04:30 AM-04:45 AM]	1	1	0	0	0	0	0	0	1
2021-07-28 [04:45 AM-05:00 AM]	2	1	1	0	0	0	0	0	2
	6	5	1	0	0	0	0	0	6
2021-07-28 [05:00 AM-05:15 AM]	7	4	2	0	0	0	0	0	6
2021-07-28 [05:15 AM-05:30 AM]	3	0	3	0	0	0	0	0	3
2021-07-28 [05:30 AM-05:45 AM]	7	4	3	0	0	0	0	0	7
2021-07-28 [05:45 AM-06:00 AM]	7	4	3	0	0	0	0	0	7
	24	12	11	0	0	0	0	0	23
2021-07-28 [06:00 AM-06:15 AM]	10	5	5	0	0	0	0	0	10
2021-07-28 [06:15 AM-06:30 AM]	13	6	7	0	0	0	0	0	13
2021-07-28 [06:30 AM-06:45 AM]	15	5	10	0	0	0	0	0	15
2021-07-28 [06:45 AM-07:00 AM]	12	2	9	0	0	0	0	0	11
	50	18	31	0	0	0	0	0	49
2021-07-28 [07:00 AM-07:15 AM]	12	3	9	0	0	0	0	0	12
2021-07-28 [07:15 AM-07:30 AM]	17	7	9	0	1	0	0	0	17
2021-07-28 [07:30 AM-07:45 AM]	26	11	12	1	2	0	0	0	26
2021-07-28 [07:45 AM-08:00 AM]	25	11	12	0	0	1	0	0	24
	80	32	42	1	3	1	0	0	79
2021-07-28 [08:00 AM-08:15 AM]	22	15	6	0	0	0	1	0	22
2021-07-28 [08:15 AM-08:30 AM]	18	9	5	2	2	0	0	0	18
2021-07-28 [08:30 AM-08:45 AM]	16	7	7	0	1	1	0	0	16
2021-07-28 [08:45 AM-09:00 AM]	23	8	14	0	0	1	0	0	23
	79	39	32	2	3	2	1	0	79
2021-07-28 [09:00 AM-09:15 AM]	21	6	14	0	1	0	0	0	21
2021-07-28 [09:15 AM-09:30 AM]	26	8	16	0	2	0	0	0	26
2021-07-28 [09:30 AM-09:45 AM]	23	14	9	0	0	0	0	0	23
2021-07-28 [09:45 AM-10:00 AM]	19	6	12	0	1	0	0	0	19

	89	34	51	0	4	0	0	0	0	89
2021-07-28 [10:00 AM-10:15 AM]	27	14	13	0	0	0	0	0	0	27
2021-07-28 [10:15 AM-10:30 AM]	16	9	6	0	0	1	0	0	0	16
2021-07-28 [10:30 AM-10:45 AM]	26	13	13	0	0	0	0	0	0	26
2021-07-28 [10:45 AM-11:00 AM]	25	15	9	0	0	1	0	0	0	25
	94	51	41	0	0	2	0	0	0	94
2021-07-28 [11:00 AM-11:15 AM]	18	7	11	0	0	0	0	0	0	18
2021-07-28 [11:15 AM-11:30 AM]	11	4	7	0	0	0	0	0	0	11
2021-07-28 [11:30 AM-11:45 AM]	0	0	0	0	0	0	0	0	0	0
2021-07-28 [11:45 AM-12:00 PM]	0	0	0	0	0	0	0	0	0	0
	29	11	18	0	0	0	0	0	0	29
2021-07-28 [12:00 PM-12:15 PM]	22	13	8	1	0	0	0	0	0	22
2021-07-28 [12:15 PM-12:30 PM]	17	10	7	0	0	0	0	0	0	17
2021-07-28 [12:30 PM-12:45 PM]	22	7	15	0	0	0	0	0	0	22
2021-07-28 [12:45 PM-01:00 PM]	19	10	7	0	2	0	0	0	0	19
	80	40	37	1	2	0	0	0	0	80
2021-07-28 [01:00 PM-01:15 PM]	23	11	12	0	0	0	0	0	0	23
2021-07-28 [01:15 PM-01:30 PM]	24	6	17	0	1	0	0	0	0	24
2021-07-28 [01:30 PM-01:45 PM]	19	5	12	1	1	0	0	0	0	19
2021-07-28 [01:45 PM-02:00 PM]	22	12	9	0	1	0	0	0	0	22
	88	34	50	1	3	0	0	0	0	88
2021-07-28 [02:00 PM-02:15 PM]	27	4	22	0	1	0	0	0	0	27
2021-07-28 [02:15 PM-02:30 PM]	17	4	12	0	0	1	0	0	0	17
2021-07-28 [02:30 PM-02:45 PM]	31	12	18	0	1	0	0	0	0	31
2021-07-28 [02:45 PM-03:00 PM]	26	9	14	1	2	0	0	0	0	26
	101	29	66	1	4	1	0	0	0	101
2021-07-28 [03:00 PM-03:15 PM]	15	5	9	0	0	1	0	0	0	15
2021-07-28 [03:15 PM-03:30 PM]	23	13	10	0	0	0	0	0	0	23
2021-07-28 [03:30 PM-03:45 PM]	27	10	15	1	0	0	0	0	0	26
2021-07-28 [03:45 PM-04:00 PM]	21	8	13	0	0	0	0	0	0	21
	86	36	47	1	0	1	0	0	0	85
2021-07-28 [04:00 PM-04:15 PM]	24	13	9	0	2	0	0	0	0	24
2021-07-28 [04:15 PM-04:30 PM]	19	10	8	0	1	0	0	0	0	19
2021-07-28 [04:30 PM-04:45 PM]	14	7	7	0	0	0	0	0	0	14
2021-07-28 [04:45 PM-05:00 PM]	20	8	12	0	0	0	0	0	0	20
	77	38	36	0	3	0	0	0	0	77
2021-07-28 [05:00 PM-05:15 PM]	22	5	16	0	1	0	0	0	0	22
2021-07-28 [05:15 PM-05:30 PM]	16	7	9	0	0	0	0	0	0	16
2021-07-28 [05:30 PM-05:45 PM]	13	7	6	0	0	0	0	0	0	13
2021-07-28 [05:45 PM-06:00 PM]	26	10	15	0	1	0	0	0	0	26
	77	29	46	0	2	0	0	0	0	77
2021-07-28 [06:00 PM-06:15 PM]	14	5	9	0	0	0	0	0	0	14
2021-07-28 [06:15 PM-06:30 PM]	16	8	8	0	0	0	0	0	0	16
2021-07-28 [06:30 PM-06:45 PM]	12	5	7	0	0	0	0	0	0	12
2021-07-28 [06:45 PM-07:00 PM]	15	7	8	0	0	0	0	0	0	15
	57	25	32	0	0	0	0	0	0	57
2021-07-28 [07:00 PM-07:15 PM]	13	6	6	0	1	0	0	0	0	13
2021-07-28 [07:15 PM-07:30 PM]	15	7	8	0	0	0	0	0	0	15
2021-07-28 [07:30 PM-07:45 PM]	9	3	6	0	0	0	0	0	0	9
2021-07-28 [07:45 PM-08:00 PM]	16	7	9	0	0	0	0	0	0	16
	53	23	29	0	1	0	0	0	0	53
2021-07-28 [08:00 PM-08:15 PM]	10	3	7	0	0	0	0	0	0	10
2021-07-28 [08:15 PM-08:30 PM]	5	2	3	0	0	0	0	0	0	5
2021-07-28 [08:30 PM-08:45 PM]	12	7	5	0	0	0	0	0	0	12
2021-07-28 [08:45 PM-09:00 PM]	6	2	4	0	0	0	0	0	0	6
	33	14	19	0	0	0	0	0	0	33

2021-07-28 [09:00 PM-09:15 PM]	4	1	3	0	0	0	0	0	0	4	
2021-07-28 [09:15 PM-09:30 PM]	3	1	2	0	0	0	0	0	0	3	
2021-07-28 [09:30 PM-09:45 PM]	3	2	1	0	0	0	0	0	0	3	
2021-07-28 [09:45 PM-10:00 PM]	4	3	1	0	0	0	0	0	0	4	
	14	7	7	0	0	0	0	0	0	14	
2021-07-28 [10:00 PM-10:15 PM]	8	3	3	0	2	0	0	0	0	8	
2021-07-28 [10:15 PM-10:30 PM]	1	0	1	0	0	0	0	0	0	1	
2021-07-28 [10:30 PM-10:45 PM]	0	0	0	0	0	0	0	0	0	0	
2021-07-28 [10:45 PM-11:00 PM]	2	0	2	0	0	0	0	0	0	2	
	11	3	6	0	2	0	0	0	0	11	
2021-07-28 [11:00 PM-11:15 PM]	6	3	3	0	0	0	0	0	0	6	
2021-07-28 [11:15 PM-11:30 PM]	1	0	1	0	0	0	0	0	0	1	
2021-07-28 [11:30 PM-11:45 PM]	1	0	1	0	0	0	0	0	0	1	
2021-07-28 [11:45 PM-12:00 AM]	1	0	1	0	0	0	0	0	0	1	
	9	3	6	0	0	0	0	0	0	9	
Daily Totals:	1140	485	609	7	27	7	1	0	0	1136	
Total Counted:	1140										
Total Classified:	1136	1140	485	609	7	27	7	1	0	1136	
Total Unclassified:	4										
Report Percentages:				42.69%	53.61%	0.62%	2.38%	0.62%	0.09%	0.00%	0.00%
Peak Time: (AM):	2021-07-28 [10:00 AM-10:15 AM]			Peak Count:	27						
Peak Time: (PM):	2021-07-28 [02:30 PM-02:45 PM]			Peak Count:	31						

### Class/Volume Report Graph

Hi-Star ID: 61B35  
 Street: 613 Helena St - SB  
 State: ON  
 City: Fort Erie  
 Area:

Begin: 2021-07-28 12:00 AM  
 Lane: SB  
 Oper: MD  
 Posted: 70  
 AADT Factor: 1  
 End: 2021-07-29 12:00 AM  
 Hours: 24:00  
 Period: 15  
 Raw Count: 1284  
 AADT Count: 1284

NC300 - Metres	0.0 to 4.9	5.0 to 8.4	8.5 to 9.9	10.0 to 12.5	13.0 to 15.5	16.0 to 18.5	19.0 to 22.4	22.5 >	Total
2021-07-28 [12:00 AM-12:15 AM]	4	2	2	0	0	0	0	0	4
2021-07-28 [12:15 AM-12:30 AM]	2	2	0	0	0	0	0	0	2
2021-07-28 [12:30 AM-12:45 AM]	1	1	0	0	0	0	0	0	1
2021-07-28 [12:45 AM-01:00 AM]	4	3	1	0	0	0	0	0	4
	11	8	3	0	0	0	0	0	11
2021-07-28 [01:00 AM-01:15 AM]	1	0	1	0	0	0	0	0	1
2021-07-28 [01:15 AM-01:30 AM]	1	1	0	0	0	0	0	0	1
2021-07-28 [01:30 AM-01:45 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [01:45 AM-02:00 AM]	0	0	0	0	0	0	0	0	0
	2	1	1	0	0	0	0	0	2
2021-07-28 [02:00 AM-02:15 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [02:15 AM-02:30 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [02:30 AM-02:45 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [02:45 AM-03:00 AM]	0	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0	0
2021-07-28 [03:00 AM-03:15 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [03:15 AM-03:30 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [03:30 AM-03:45 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [03:45 AM-04:00 AM]	0	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0	0
2021-07-28 [04:00 AM-04:15 AM]	1	1	0	0	0	0	0	0	1
2021-07-28 [04:15 AM-04:30 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [04:30 AM-04:45 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [04:45 AM-05:00 AM]	0	0	0	0	0	0	0	0	0
	1	1	0	0	0	0	0	0	1
2021-07-28 [05:00 AM-05:15 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [05:15 AM-05:30 AM]	1	0	1	0	0	0	0	0	1
2021-07-28 [05:30 AM-05:45 AM]	2	2	0	0	0	0	0	0	2
2021-07-28 [05:45 AM-06:00 AM]	2	2	0	0	0	0	0	0	2
	5	4	1	0	0	0	0	0	5
2021-07-28 [06:00 AM-06:15 AM]	3	1	2	0	0	0	0	0	3
2021-07-28 [06:15 AM-06:30 AM]	3	1	0	1	1	0	0	0	3
2021-07-28 [06:30 AM-06:45 AM]	0	0	0	0	0	0	0	0	0
2021-07-28 [06:45 AM-07:00 AM]	2	2	0	0	0	0	0	0	2
	8	4	2	1	1	0	0	0	8
2021-07-28 [07:00 AM-07:15 AM]	10	4	4	1	1	0	0	0	10
2021-07-28 [07:15 AM-07:30 AM]	8	4	4	0	0	0	0	0	8
2021-07-28 [07:30 AM-07:45 AM]	11	6	4	0	1	0	0	0	11
2021-07-28 [07:45 AM-08:00 AM]	10	5	4	0	0	1	0	0	10
	39	19	16	1	2	1	0	0	39
2021-07-28 [08:00 AM-08:15 AM]	10	7	2	0	1	0	0	0	10
2021-07-28 [08:15 AM-08:30 AM]	11	4	6	1	0	0	0	0	11
2021-07-28 [08:30 AM-08:45 AM]	10	6	3	0	0	1	0	0	10
2021-07-28 [08:45 AM-09:00 AM]	10	6	4	0	0	0	0	0	10
	41	23	15	1	1	1	0	0	41
2021-07-28 [09:00 AM-09:15 AM]	12	7	5	0	0	0	0	0	12
2021-07-28 [09:15 AM-09:30 AM]	16	7	9	0	0	0	0	0	16
2021-07-28 [09:30 AM-09:45 AM]	15	9	6	0	0	0	0	0	15
2021-07-28 [09:45 AM-10:00 AM]	21	10	8	1	2	0	0	0	21
	64	33	28	1	2	0	0	0	64

2021-07-28 [10:00 AM-10:15 AM]	11	5	5	0	1	0	0	0	0	11
2021-07-28 [10:15 AM-10:30 AM]	15	11	3	1	0	0	0	0	0	15
2021-07-28 [10:30 AM-10:45 AM]	26	14	11	0	1	0	0	0	0	26
2021-07-28 [10:45 AM-11:00 AM]	15	10	4	0	1	0	0	0	0	15
	67	40	23	1	3	0	0	0	0	67
2021-07-28 [11:00 AM-11:15 AM]	19	9	9	0	1	0	0	0	0	19
2021-07-28 [11:15 AM-11:30 AM]	34	21	10	1	1	0	1	0	0	34
2021-07-28 [11:30 AM-11:45 AM]	45	22	21	1	0	1	0	0	0	45
2021-07-28 [11:45 AM-12:00 PM]	37	15	18	1	3	0	0	0	0	37
	135	67	58	3	5	1	1	0	0	135
2021-07-28 [12:00 PM-12:15 PM]	28	11	13	2	1	0	0	1	0	28
2021-07-28 [12:15 PM-12:30 PM]	23	11	12	0	0	0	0	0	0	23
2021-07-28 [12:30 PM-12:45 PM]	22	8	10	1	2	1	0	0	0	22
2021-07-28 [12:45 PM-01:00 PM]	17	10	7	0	0	0	0	0	0	17
	90	40	42	3	3	1	0	1	0	90
2021-07-28 [01:00 PM-01:15 PM]	19	11	8	0	0	0	0	0	0	19
2021-07-28 [01:15 PM-01:30 PM]	24	15	7	1	1	0	0	0	0	24
2021-07-28 [01:30 PM-01:45 PM]	26	15	9	0	1	1	0	0	0	26
2021-07-28 [01:45 PM-02:00 PM]	29	19	9	0	1	0	0	0	0	29
	98	60	33	1	3	1	0	0	0	98
2021-07-28 [02:00 PM-02:15 PM]	17	11	6	0	0	0	0	0	0	17
2021-07-28 [02:15 PM-02:30 PM]	29	12	15	0	1	0	0	0	0	28
2021-07-28 [02:30 PM-02:45 PM]	25	7	18	0	0	0	0	0	0	25
2021-07-28 [02:45 PM-03:00 PM]	32	19	12	0	0	0	0	1	0	32
	103	49	51	0	1	0	0	1	0	102
2021-07-28 [03:00 PM-03:15 PM]	19	12	7	0	0	0	0	0	0	19
2021-07-28 [03:15 PM-03:30 PM]	27	15	12	0	0	0	0	0	0	27
2021-07-28 [03:30 PM-03:45 PM]	21	11	8	1	1	0	0	0	0	21
2021-07-28 [03:45 PM-04:00 PM]	41	27	14	0	0	0	0	0	0	41
	108	65	41	1	1	0	0	0	0	108
2021-07-28 [04:00 PM-04:15 PM]	34	22	11	0	1	0	0	0	0	34
2021-07-28 [04:15 PM-04:30 PM]	37	19	18	0	0	0	0	0	0	37
2021-07-28 [04:30 PM-04:45 PM]	24	11	13	0	0	0	0	0	0	24
2021-07-28 [04:45 PM-05:00 PM]	27	16	10	0	1	0	0	0	0	27
	122	68	52	0	2	0	0	0	0	122
2021-07-28 [05:00 PM-05:15 PM]	46	28	17	1	0	0	0	0	0	46
2021-07-28 [05:15 PM-05:30 PM]	32	23	9	0	0	0	0	0	0	32
2021-07-28 [05:30 PM-05:45 PM]	31	19	12	0	0	0	0	0	0	31
2021-07-28 [05:45 PM-06:00 PM]	28	15	13	0	0	0	0	0	0	28
	137	85	51	1	0	0	0	0	0	137
2021-07-28 [06:00 PM-06:15 PM]	17	8	9	0	0	0	0	0	0	17
2021-07-28 [06:15 PM-06:30 PM]	15	11	4	0	0	0	0	0	0	15
2021-07-28 [06:30 PM-06:45 PM]	17	11	6	0	0	0	0	0	0	17
2021-07-28 [06:45 PM-07:00 PM]	11	5	6	0	0	0	0	0	0	11
	60	35	25	0	0	0	0	0	0	60
2021-07-28 [07:00 PM-07:15 PM]	4	0	3	0	1	0	0	0	0	4
2021-07-28 [07:15 PM-07:30 PM]	19	9	10	0	0	0	0	0	0	19
2021-07-28 [07:30 PM-07:45 PM]	24	14	10	0	0	0	0	0	0	24
2021-07-28 [07:45 PM-08:00 PM]	15	8	6	0	1	0	0	0	0	15
	62	31	29	0	2	0	0	0	0	62
2021-07-28 [08:00 PM-08:15 PM]	12	9	3	0	0	0	0	0	0	12
2021-07-28 [08:15 PM-08:30 PM]	15	11	4	0	0	0	0	0	0	15
2021-07-28 [08:30 PM-08:45 PM]	17	8	9	0	0	0	0	0	0	17
2021-07-28 [08:45 PM-09:00 PM]	18	10	8	0	0	0	0	0	0	18
	62	38	24	0	0	0	0	0	0	62
2021-07-28 [09:00 PM-09:15 PM]	15	9	6	0	0	0	0	0	0	15
2021-07-28 [09:15 PM-09:30 PM]	8	6	2	0	0	0	0	0	0	8

2021-07-28 [09:30 PM-09:45 PM]	4	4	0	0	0	0	0	0	0	4
2021-07-28 [09:45 PM-10:00 PM]	3	1	2	0	0	0	0	0	0	3
	30	20	10	0	0	0	0	0	0	30
2021-07-28 [10:00 PM-10:15 PM]	9	5	4	0	0	0	0	0	0	9
2021-07-28 [10:15 PM-10:30 PM]	9	4	5	0	0	0	0	0	0	9
2021-07-28 [10:30 PM-10:45 PM]	6	4	2	0	0	0	0	0	0	6
2021-07-28 [10:45 PM-11:00 PM]	4	1	3	0	0	0	0	0	0	4
	28	14	14	0	0	0	0	0	0	28
2021-07-28 [11:00 PM-11:15 PM]	4	2	2	0	0	0	0	0	0	4
2021-07-28 [11:15 PM-11:30 PM]	2	2	0	0	0	0	0	0	0	2
2021-07-28 [11:30 PM-11:45 PM]	2	2	0	0	0	0	0	0	0	2
2021-07-28 [11:45 PM-12:00 AM]	3	2	1	0	0	0	0	0	0	3
	11	8	3	0	0	0	0	0	0	11
Daily Totals:	1284	713	522	14	26	5	1	2	0	1283
Total Counted:	1284									
Total Classified:	1283	1284	713	522	14	26	5	1	2	0
Total Unclassified:	1									1283
Report Percentages:		55.57%	40.69%	1.09%	2.03%	0.39%	0.08%	0.16%	0.00%	
Peak Time: (AM):	2021-07-28 [11:30 AM-11:45 AM]		Peak Count:	45						
Peak Time: (PM):	2021-07-28 [05:00 PM-05:15 PM]		Peak Count:	46						



2021-07-28 [03:00 PM-03:15 PM]	0	0	0	1	0	1	4	6	3	0	0	0	0	0	0	15
2021-07-28 [03:15 PM-03:30 PM]	0	0	0	0	0	3	2	9	9	0	0	0	0	0	0	23
2021-07-28 [03:30 PM-03:45 PM]	0	0	1	0	0	0	3	9	7	5	1	0	0	0	0	26
2021-07-28 [03:45 PM-04:00 PM]	0	0	0	0	0	0	2	7	10	2	0	0	0	0	0	21
	0	0	1	1	0	9	22	32	19	1	0	0	0	0	0	85
2021-07-28 [04:00 PM-04:15 PM]	0	0	0	0	0	8	5	8	2	0	1	0	0	0	0	24
2021-07-28 [04:15 PM-04:30 PM]	0	0	0	0	0	1	5	6	5	1	0	0	1	0	0	19
2021-07-28 [04:30 PM-04:45 PM]	0	0	0	0	0	0	5	4	4	1	0	0	0	0	0	14
2021-07-28 [04:45 PM-05:00 PM]	0	0	0	0	1	1	5	8	4	1	0	0	0	0	0	20
	0	0	0	0	1	10	20	26	15	3	1	0	1	0	0	77
2021-07-28 [05:00 PM-05:15 PM]	0	0	0	0	0	1	7	8	5	1	0	0	0	0	0	22
2021-07-28 [05:15 PM-05:30 PM]	0	0	0	0	0	4	6	1	3	2	0	0	0	0	0	16
2021-07-28 [05:30 PM-05:45 PM]	0	0	0	0	0	0	2	8	1	2	0	0	0	0	0	13
2021-07-28 [05:45 PM-06:00 PM]	0	0	0	0	0	4	4	10	5	3	0	0	0	0	0	26
	0	0	0	0	0	9	19	27	14	8	0	0	0	0	0	77
2021-07-28 [06:00 PM-06:15 PM]	0	0	0	0	0	3	3	3	5	0	0	0	0	0	0	14
2021-07-28 [06:15 PM-06:30 PM]	0	0	0	0	0	1	5	7	3	0	0	0	0	0	0	16
2021-07-28 [06:30 PM-06:45 PM]	0	0	0	0	1	0	4	3	2	1	1	0	0	0	0	12
2021-07-28 [06:45 PM-07:00 PM]	0	0	0	0	0	1	4	6	3	1	0	0	0	0	0	15
	0	0	0	0	1	5	16	19	13	2	1	0	0	0	0	57
2021-07-28 [07:00 PM-07:15 PM]	0	0	0	0	0	0	3	4	3	2	0	0	1	0	0	13
2021-07-28 [07:15 PM-07:30 PM]	0	0	0	0	0	1	2	6	5	1	0	0	0	0	0	15
2021-07-28 [07:30 PM-07:45 PM]	0	0	0	0	0	0	0	1	2	3	2	1	0	0	0	9
2021-07-28 [07:45 PM-08:00 PM]	0	0	0	0	0	1	3	9	0	2	0	1	0	0	0	16
	0	0	0	0	0	2	9	21	11	7	1	1	1	0	0	53
2021-07-28 [08:00 PM-08:15 PM]	0	0	0	0	0	0	3	2	2	1	1	1	0	0	0	10
2021-07-28 [08:15 PM-08:30 PM]	0	0	0	0	0	0	1	3	0	1	0	0	0	0	0	5
2021-07-28 [08:30 PM-08:45 PM]	0	0	0	0	0	2	2	7	1	0	0	0	0	0	0	12
2021-07-28 [08:45 PM-09:00 PM]	0	0	0	0	0	0	2	4	0	0	0	0	0	0	0	6
	0	0	0	0	0	2	8	16	3	2	1	1	0	0	0	33
2021-07-28 [09:00 PM-09:15 PM]	0	0	0	0	0	1	2	1	0	0	0	0	0	0	0	4
2021-07-28 [09:15 PM-09:30 PM]	0	0	0	0	0	0	0	1	0	1	1	0	0	0	0	3
2021-07-28 [09:30 PM-09:45 PM]	0	0	0	0	0	0	0	1	1	1	0	0	0	0	0	3
2021-07-28 [09:45 PM-10:00 PM]	0	0	0	0	0	1	2	1	0	0	0	0	0	0	0	4
	0	0	0	0	0	2	6	3	2	1	0	0	0	0	0	14
2021-07-28 [10:00 PM-10:15 PM]	0	0	0	0	0	0	4	1	2	1	0	0	0	0	0	8
2021-07-28 [10:15 PM-10:30 PM]	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1
2021-07-28 [10:30 PM-10:45 PM]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2021-07-28 [10:45 PM-11:00 PM]	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2
	0	0	0	0	0	0	5	3	2	1	0	0	0	0	0	11
2021-07-28 [11:00 PM-11:15 PM]	0	0	0	0	0	1	0	3	2	0	0	0	0	0	0	6
2021-07-28 [11:15 PM-11:30 PM]	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1
2021-07-28 [11:30 PM-11:45 PM]	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1
2021-07-28 [11:45 PM-12:00 AM]	0	0	0	0	0	0	1	2	4	2	0	0	0	0	0	9
Daily Totals:	0	3	2	3	18	118	321	373	191	76	15	9	7	0	0	1136
Report Totals:	0	3	2	3	18	118	321	373	191	76	15	9	7	0	0	1136
Report Percentages:	0.00%	0.26%	0.18%	0.26%	1.58%	10.39%	28.26%	32.83%	16.81%	6.69%	1.32%	0.79%	0.62%	0.00%	0.00%	



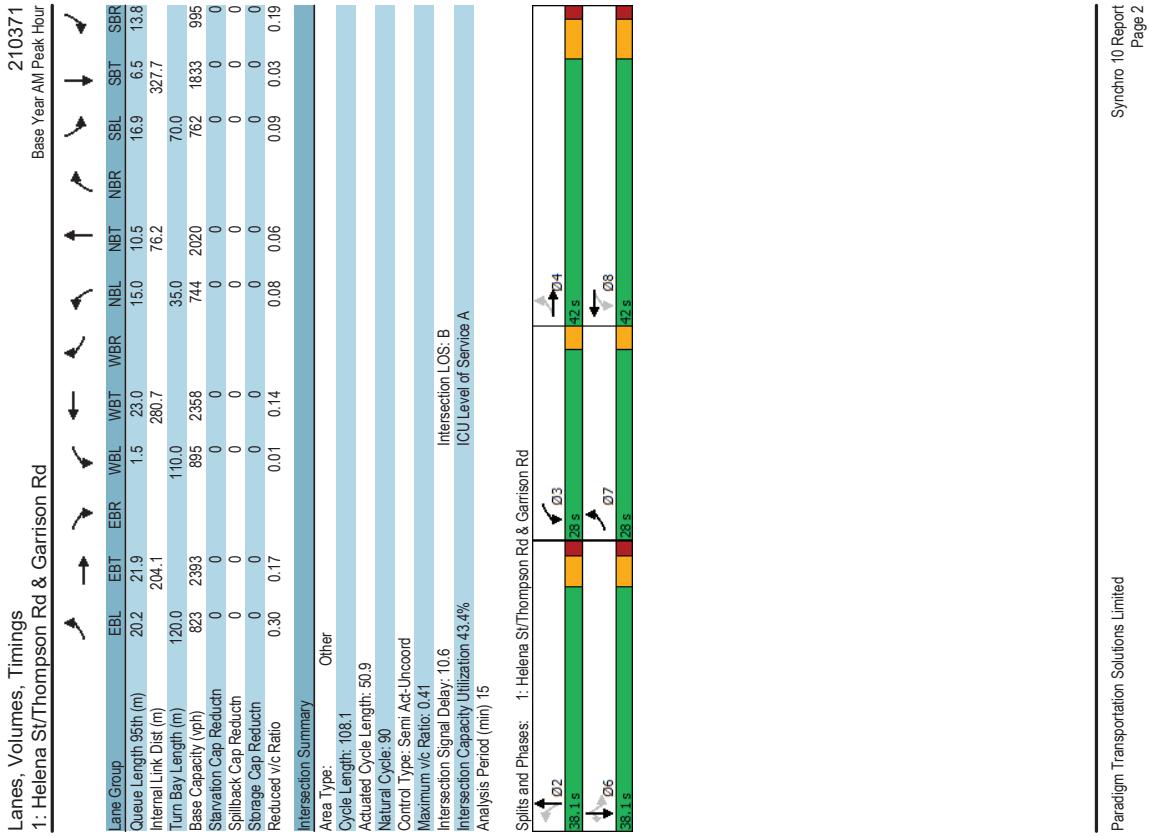
2021-07-28 [03:00 PM-03:15 PM]	0	0	0	0	0	0	3	8	7	1	0	0	0	0	0	19
2021-07-28 [03:15 PM-03:30 PM]	0	0	0	0	0	3	5	14	3	0	1	0	1	0	0	27
2021-07-28 [03:30 PM-03:45 PM]	0	0	0	0	0	2	8	7	3	1	0	0	0	0	0	21
2021-07-28 [03:45 PM-04:00 PM]	0	0	0	0	1	2	7	23	5	0	2	1	0	0	0	41
	0	0	0	0	1	7	23	52	18	2	3	1	1	0	0	108
2021-07-28 [04:00 PM-04:15 PM]	0	1	2	0	0	4	14	12	0	1	0	0	0	0	0	34
2021-07-28 [04:15 PM-04:30 PM]	0	0	0	0	0	1	10	15	9	2	0	0	0	0	0	37
2021-07-28 [04:30 PM-04:45 PM]	0	0	0	0	0	0	7	7	7	1	2	0	0	0	0	24
2021-07-28 [04:45 PM-05:00 PM]	0	0	0	0	0	0	8	11	5	1	2	0	0	0	0	27
	0	1	2	0	0	5	39	45	21	5	4	0	0	0	0	122
2021-07-28 [05:00 PM-05:15 PM]	0	0	0	0	0	1	13	21	9	2	0	0	0	0	0	46
2021-07-28 [05:15 PM-05:30 PM]	0	0	0	0	0	2	11	16	1	2	0	0	0	0	0	32
2021-07-28 [05:30 PM-05:45 PM]	0	0	0	0	0	1	8	11	3	8	0	0	0	0	0	31
2021-07-28 [05:45 PM-06:00 PM]	0	0	0	0	1	3	8	9	4	3	0	0	0	0	0	28
	0	0	0	0	1	7	40	57	17	15	0	0	0	0	0	137
2021-07-28 [06:00 PM-06:15 PM]	0	0	0	0	0	1	3	8	4	1	0	0	0	0	0	17
2021-07-28 [06:15 PM-06:30 PM]	0	0	0	0	0	2	4	6	2	1	0	0	0	0	0	15
2021-07-28 [06:30 PM-06:45 PM]	0	0	0	0	0	1	3	9	1	2	1	0	0	0	0	17
2021-07-28 [06:45 PM-07:00 PM]	0	0	0	0	1	0	0	2	4	3	0	1	0	0	0	11
	0	0	0	0	1	0	4	12	27	10	4	2	0	0	0	60
2021-07-28 [07:00 PM-07:15 PM]	0	0	0	0	0	0	0	2	2	0	0	0	0	0	0	4
2021-07-28 [07:15 PM-07:30 PM]	0	0	0	0	0	2	5	7	3	1	1	0	0	0	0	19
2021-07-28 [07:30 PM-07:45 PM]	0	0	0	0	0	1	10	7	2	4	0	0	0	0	0	24
2021-07-28 [07:45 PM-08:00 PM]	0	0	0	0	0	0	5	7	2	1	0	0	0	0	0	15
	0	0	0	0	0	3	20	23	9	6	1	0	0	0	0	62
2021-07-28 [08:00 PM-08:15 PM]	0	0	0	0	1	0	9	0	2	0	0	0	0	0	0	12
2021-07-28 [08:15 PM-08:30 PM]	0	0	0	0	0	3	6	3	0	2	1	0	0	0	0	15
2021-07-28 [08:30 PM-08:45 PM]	0	0	0	0	0	1	4	9	2	1	0	0	0	0	0	17
2021-07-28 [08:45 PM-09:00 PM]	0	0	0	0	0	1	3	6	5	3	0	0	0	0	0	18
	0	0	0	0	1	5	22	18	9	6	1	0	0	0	0	62
2021-07-28 [09:00 PM-09:15 PM]	0	0	0	0	0	0	5	4	2	4	0	0	0	0	0	15
2021-07-28 [09:15 PM-09:30 PM]	0	0	0	0	0	1	1	4	1	1	0	0	0	0	0	8
2021-07-28 [09:30 PM-09:45 PM]	0	0	0	0	0	0	1	3	0	0	0	0	0	0	0	4
2021-07-28 [09:45 PM-10:00 PM]	0	0	0	0	0	0	0	2	0	0	1	0	0	0	0	3
	0	0	0	0	0	1	7	13	3	5	1	0	0	0	0	30
2021-07-28 [10:00 PM-10:15 PM]	0	0	0	0	0	1	0	6	2	0	0	0	0	0	0	9
2021-07-28 [10:15 PM-10:30 PM]	0	0	0	0	0	0	3	2	2	2	0	0	0	0	0	9
2021-07-28 [10:30 PM-10:45 PM]	0	0	0	0	0	2	0	3	1	0	0	0	0	0	0	6
2021-07-28 [10:45 PM-11:00 PM]	0	0	0	0	0	0	1	0	1	1	1	0	0	0	0	4
	0	0	0	0	0	3	4	11	6	3	1	0	0	0	0	28
2021-07-28 [11:00 PM-11:15 PM]	0	0	0	0	0	0	1	0	2	0	0	0	1	0	0	4
2021-07-28 [11:15 PM-11:30 PM]	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2
2021-07-28 [11:30 PM-11:45 PM]	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	2
2021-07-28 [11:45 PM-12:00 AM]	0	0	0	0	0	0	1	1	1	0	0	0	0	0	0	3
	0	0	0	0	0	2	2	5	1	0	0	1	0	0	0	11
Daily Totals:	0	2	3	2	12	105	355	494	195	82	27	3	3	0	0	1283
Report Totals:	0	2	3	2	12	105	355	494	195	82	27	3	3	0	0	1283
Report Percentages:	0.00%	0.16%	0.23%	0.16%	0.94%	8.18%	27.67%	38.50%	15.20%	6.39%	2.10%	0.23%	0.23%	0.00%	0.00%	

## Appendix C

### Base Year Traffic Operations



Lanes, Volumes, Timings 1: Helena St/Thompson Rd & Garrison Rd							210371 Base Year AM Peak Hour						
Lane Group	EBL	EFT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	SBT
Lane Configurations	229	354	24	7	240	53	57	91	13	66	49	176	176
Traffic Volume (vph)	229	354	24	7	240	53	57	91	13	66	49	176	327.7
Ideal Flow (vph)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	995
Storage Length (m)	1200	0	110.0	0.0	35.0	0.0	70.0	0.0	70.0	0.0	0.0	0.0	0.0
Storage Lanes	1	0	1	0	1	0	1	0	1	0	0	0	0
Taper Length (m)	7.5	95	1.00	0.95	1.00	0.95	0.95	1.00	0.95	0.95	1.00	0.95	0.19
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	0.95	1.00	0.95	0.95	1.00	0.95	0.09
Fit	0.950	0.991	0.950	0.973	0.950	0.981	0.950	0.961	0.950	0.950	0.950	0.950	0.850
Fit Protected	0.950	15.64	0	16.62	31.11	0	14.46	29.72	0	15.68	27.03	13.77	
Satd. Flow (prot)	0.432	707	3164	0	854	3111	0	1097	2972	0	1124	2703	1377
Fit Permitted	0.432	707	3164	0	854	3111	0	1097	2972	0	1124	2703	1377
Satd. Flow (perm)	0.432	707	3164	0	854	3111	0	1097	2972	0	1124	2703	1377
Right Turn on Red	Yes						Yes					Yes	
Satd. Flow (RTOR)	7						27					14	
Link Speed (km/h)	60						60					70	
Link Distance (m)	228.1						304.7					100.2	
Travel Time (s)	13.7						18.3					5.2	
Peak Hour Factor	0.92						0.92					0.92	
Heavy Vehicles (%)	7%						21%					4%	
Adj. Flow (vph)	249						385					15%	
Shared Lane Traffic (%)												10%	
Lane Group Flow (vph)	249						411					89	
Turn Type	pm+pt						NA					6%	
Protected Phases	7						4					28%	
Permitted Phases	4						8					8%	
Detector Phase	7						4					191	
Switch Phase							3					191	
Minimum Initial (s)	6.0						10.0					10.0	
Minimum Split (s)	9.0						9.0					10.0	
Total Split (s)	28.0						42.0					38.1	
Total Split (%)	25.9%						38.9%					38.1	
Yellow Time (s)	3.0						5.0					38.1	
AIR/Red Time (s)	0.0						2.0					38.1	
Lost Time Adjust (s)	1.0						3.0					38.1	
Total Lost Time (s)	4.0						4.0					38.1	
Lead/Lag	Lead						Lag					38.1	
Lead-Lag Optimize?	None						Min					38.1	
Recall Mode	None						Min					38.1	
Act. Efect Green (s)	30.1						28.6					38.1	
Actuated g/c Ratio	0.59						0.56					38.1	
vic Ratio	0.41						0.23					38.1	
Control Delay	7.2						6.5					38.1	
Queue Delay	0.0						0.0					38.1	
Total Delay	7.2						6.5					38.1	
LOS	A						A					38.1	
Approach Delay	6.8						A					38.1	
Approach LOS	A						B					38.1	
Queue Length 50th (m)	9.3						7.6					38.1	



Lanes, Volumes, Timings														
2: Helena St & Washington Rd/Albany St														
Base Year AM Peak Hour														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT			
Lane Configurations														
Traffic Volume (vph)	229	354	24	7	240	53	57	91	13	66	49	176	43	3
Future Volume (vph)	229	354	24	7	240	53	57	91	13	66	49	176	43	3
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Losttime (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Fit	1.00	0.98	1.00	0.97	1.00	0.98	1.00	0.98	1.00	0.98	1.00	0.98	1.00	0.98
Fit Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1554	3163	1662	3110	1446	2973	1568	2703	1377					
Fit Permitted	0.43	1.00	0.51	1.00	0.72	1.00	0.68	1.00	0.68	1.00	0.68	1.00	0.68	1.00
Satd. Flow (perm)	707	3163	834	3110	1097	2973	1124	2703	1377					
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	249	385	26	8	261	58	62	99	14	72	53	191	25.6	23.0
R/TOR Reduction (vph)	0	3	0	0	0	18	0	0	11	0	0	0	0.92	0.92
Lane Group Flow (vph)	249	408	0	8	301	0	62	102	0	72	53	46	0%	25%
Heavy Vehicles (%)	7%	3%	21%	0%	4%	4%	15%	10%	8%	6%	23%	8%		
Turn Type	phn+pt	NA	phn+pt	NA	phn+pt	NA	phn+pt	NA	phn+pt	NA	phn+pt	NA	phn+pt	NA
Protected Phases	7	4	3	8	2	2	2	2	2	2	2	2	2	2
Permitted Phases	4	296	25.6	15.7	14.7	10.6	10.6	10.6	10.6	10.6	10.6	10.6	10.6	10.6
Actuated Green, G (s)	28.6	28.6	13.7	17.7	12.7	12.7	12.7	12.7	12.7	12.7	12.7	12.7		
Effective Green, g (s)	0.54	0.54	0.26	0.33	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24		
Actuated GC Ratio	Vehicle Extension (s)	3.0	7.0	3.0	7.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		
Lane Grip Cap (vph)	552	1697	229	1032	261	708	267	708	267	708	267	708		
VIS Ratio Prot	0.09	0.13	0.10	0.01	0.06	0.03	0.06	0.06	0.06	0.06	0.06	0.06		
VIS Ratio Perm	0.45	0.24	0.03	0.29	0.24	0.14	0.24	0.14	0.24	0.24	0.24	0.24		
VIS Ratio	Uniform Delay, d1	7.0	6.6	14.8	13.2	16.4	16.0	16.5	15.8	16.0				
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
Incremental Delay, d2	0.4	0.2	0.0	0.3	0.5	0.1	0.5	0.1	0.5	0.1				
Delay (s)	7.4	6.7	14.9	13.5	16.9	16.1	17.1	15.8	16.2					
Level of Service	A	A	B	B	B	B	B	B	B	B	B	B		
Approach Delay (s)	7.0	13.5	13.5	13.5	16.4	16.4	16.4	16.4	16.4	16.4				
Approach LOS	A	A	B	B	B	B	B	B	B	B				

Intersection Summary														
HCM 2000 Control Delay														
HCM 2000 Volume to Capacity ratio														
Actualized Cycle Length (s)														
Intersection Capacity Utilization														
Analysis Period (min)														
c Critical Lane Group														
HCM 2000 Control Delay	11.5	HCM 2000 Level of Service												
HCM 2000 Volume to Capacity ratio	0.39	Sum of lost time (s)												
Actualized Cycle Length (s)	53.3	ICU Level of Service												
Intersection Capacity Utilization	43.4%	A												
Analysis Period (min)	15													

HCM Unsignalized Intersection Capacity Analysis  
2: Helena St & Washington Rd/Albany St

210371  
Base Year AM Peak Hour

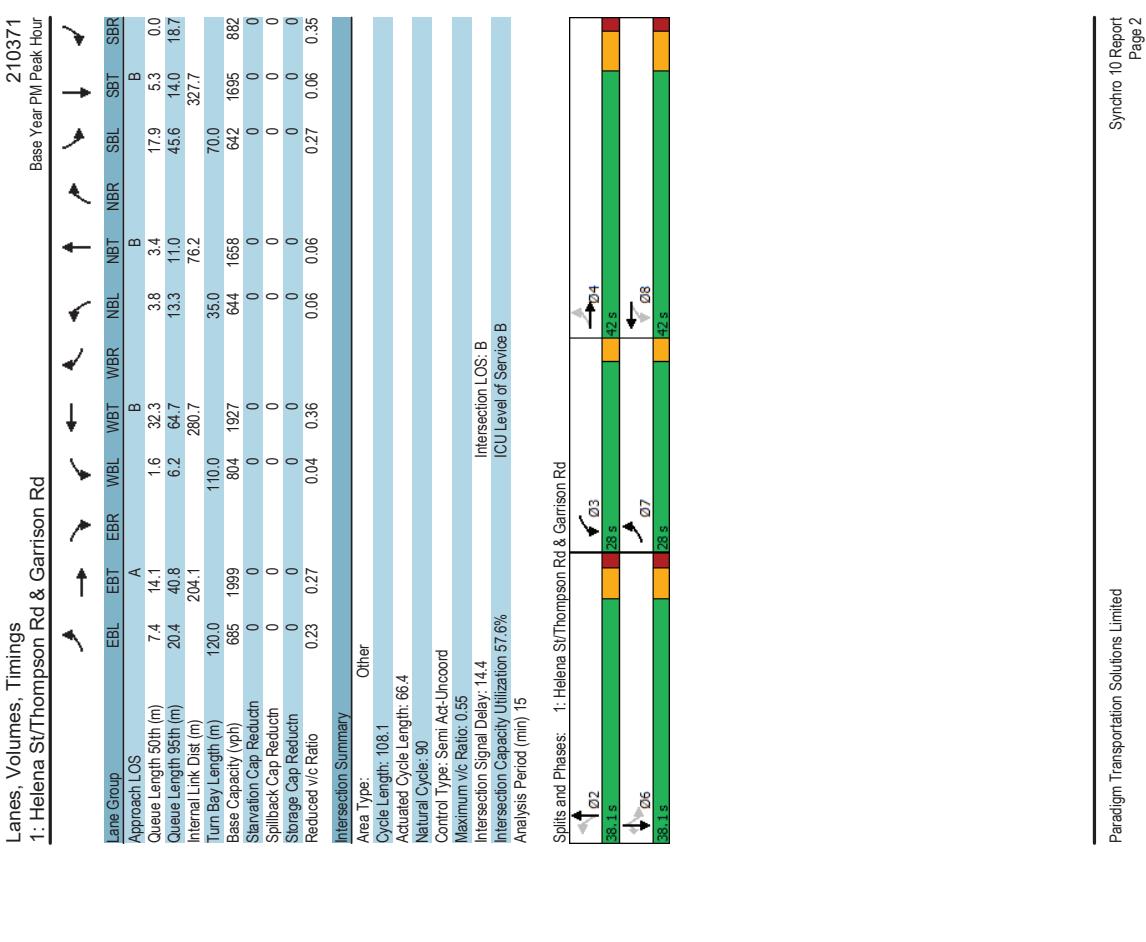
Movement	EBL	EBT	EBC	WBL	WBT	WBC	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												Stop
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Volume (vph)	25	21	0	4	18	43	3	39	2	12	13	4
Future Volume (vph)	25	21	0	4	18	43	3	39	2	12	13	4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	27	23	0	4	20	47	3	42	2	13	14	4
Direction Lane #	EB1	WB1	NB1	SB1								
Volume Left (vph)	50	71	47	31								
Volume Right (vph)	0	47	2	4								
Hadj (s)	0.15	-0.28	0.06	0.12								
Departure Headway (s)	4.3	3.8	4.2	4.3								
Degree Utilization, x	0.06	0.08	0.06	0.04								
Capacity (veh/h)	817	911	814	805								
Control Delay (s)	7.6	7.2	7.5	7.5								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay	7.4											
Level of Service	A											
Intersection Capacity Utilization	21.9%											
Analysis Period (min)	15											
ICU Level of Service	A											

Queuing and Blocking Report

210371  
Base Year AM Peak Hour

Intersection: 1: Helena St/Thompson Rd & Garrison Rd												
Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	NB	NB	SB
Directions Served	L	T	TR	L	T	TR	L	T	TR	L	T	T
Maximum Queue (m)	41.7	27.0	26.8	9.4	37.1	29.2	30.7	23.4	22.2	31.7	24.5	3.0
Average Queue (m)	21.1	13.3	10.4	1.3	18.5	10.5	11.9	8.0	8.5	10.9	8.0	0.1
95th Queue (m)	36.5	24.1	21.7	6.3	31.6	21.7	25.6	18.7	18.6	23.7	20.8	2.1
Link Distance (m)	212.3	212.3	212.3	212.3	289.8	289.8	289.8	289.8	289.8	289.8	289.8	335.9
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (m)	120.0											
Storage Blk Time (%)												
Queuing Penalty (veh)												
Intersection: 2: Helena St & Washington Rd/Albany St												
Movement	EB	WB	NB	SB								
Directions Served	LTR											
Maximum Queue (m)	16.2	14.1	11.9	9.9								
Average Queue (m)	5.7	6.5	2.5	1.9								
95th Queue (m)	12.0	12.0	7.1	6.8								
Link Distance (m)	220.5	337.5	204.6	434.3								
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (m)												
Storage Blk Time (%)												
Queuing Penalty (veh)												
Zone Summary												
Zone wide Queuing Penalty	0											

Lanes, Volumes, Timings 1: Helena St/Thompson Rd & Garrison Rd												Lanes, Volumes, Timings 1: Helena St/Thompson Rd & Garrison Rd													
Base Year PM Peak Hour						210371						Base Year PM Peak Hour						210371							
Lane Group	E BL	E BT	E BR	W BL	W BT	W BR	N BL	N BT	N BR	S BL	S BT	Lane Group	E BL	E BT	E BR	W BL	W BT	W BR	N BL	N BT	N BR	S BL	S BT		
Lane Configurations	144	456	39	33	512	126	38	65	31	157	101	Approach LOS	A	B	B	3.8	3.4	17.9	5.3	0.0	0.0	0.0	0.0		
Traffic Volume (vph)	144	456	39	33	512	126	38	65	31	157	101	Queue Length 50th (m)	7.4	14.1	1.6	32.3	3.8	13.3	11.0	45.6	14.0	18.7	327.7		
Future Volume (vph)	144	456	39	33	512	126	38	65	31	157	101	Queue Length 95th (m)	20.4	40.8	6.2	64.7	3.4	17.9	18.3	45.6	14.0	18.7	327.7		
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	Internal Link Dist (m)	204.1	280.7	76.2	70.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0		
Storage Length (m)	1200	0.0	110.0	0.0	35.0	0.0	70.0	0.0	70.0	0.0	0.0	Turn Bay Length (m)	120.0	110.0	64.4	192.7	64.2	165.8	64.2	165.8	64.2	165.8	64.2		
Storage Lanes	1	0	1	0	1	0	1	0	1	0	1	Base Capacity (vph)	685	1999	804	1927	644	1658	642	165.8	642	165.8	642		
Taper Length (m)	7.5	0.95	0.95	1.00	0.95	0.95	1.00	0.95	0.95	1.00	0.95	Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0		
Ped Bike Factor	1.00	0.988	0.970	0.970	0.970	0.970	0.970	0.970	0.970	0.970	0.970	Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0		
Fit	Fit Protected	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	Reduced v/c Ratio	0.23	0.27	0.04	0.36	0.06	0.06	0.06	0.06	0.06	0.06	0.06		
Said. Flow (prot)	1568	3217	0	1662	3179	0	1662	3039	0	1646	3137	1390	Intersection Summary	Other	Area Type:	Other	Cycle Length:	108.1	Intersection LOS: B	Intersection LOS: B	Intersection LOS: B	Intersection LOS: B	Intersection LOS: B	Intersection LOS: B	
Fit Permitted	0.247	0.452	0.452	0.452	0.452	0.452	0.452	0.452	0.452	0.452	0.452	Actuated Cycle Length:	66.4	Natural Cycle:	90	Control Type:	Semi Act-Uncoord	Analysis Period (min):	15	ICU Level of Service A	ICU Level of Service B	ICU Level of Service B	ICU Level of Service B		
Said. Flow (perm)	498	3217	0	791	3179	0	1192	3039	0	1189	3137	1372	Link Distance (km)	60	60	70	70	70	70	70	70	70	70	70	
Right Turn on Red	Said. Flow (RTOR)	9	Yes	Yes	31	31	Yes	34	34	Yes	Yes	Yes	Link Distance (m)	228.1	304.7	100.2	356.7	18.1	Intersection LOS: B	Intersection LOS: B	Intersection LOS: B	Intersection LOS: B	Intersection LOS: B	Intersection LOS: B	
Link Speed (km/h)	60	60	60	60	60	60	60	60	60	60	60	Travel Time (s)	13.7	18.3	5.2	18.1	13.7	Intersection LOS: B	Intersection LOS: B	Intersection LOS: B	Intersection LOS: B	Intersection LOS: B	Intersection LOS: B		
Confli. Peds. (#/hr)	1	1	1	1	1	1	1	1	1	1	1	Confli. Peds. (#/hr)	1	1	1	1	1	Analysis Period (min):	15	Analysis Period (min):	15	Analysis Period (min):	15	Analysis Period (min):	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	Heavy Vehicles (%)	6%	2%	2%	1%	2%	0%	6%	1%	6%	7%	7%		
Adj. Flow (vph)	157	436	42	36	557	137	41	71	34	171	110	307	Shared Lane Traffic (%)	157	538	0	36	694	0	41	105	0	171	110	307
Lane Group Flow (vph)	pm+pt	NA	pm+pt	NA	pm+pt	NA	pm+pt	NA	pm+pt	NA	NA	Turn Type	7	4	3	8	8	Perm	NA	Perm	NA	Perm	NA		
Protected Phases	4	7	4	3	8	2	2	2	2	2	6	Permitted Phases	4	7	4	3	8	2	2	2	2	6	6		
Permitted Phases	7	4	3	8	2	2	2	2	2	2	6	Detector Phase	Switch Phase	02	03	02	03	02	03	02	03	02	03		
Detector Phase	Switch Phase	0.02	0.03	0.02	0.03	0.02	0.03	0.02	0.03	0.02	0.03	Minimum Initial (s)	6.0	10.0	6.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		
Minimum Split (s)	9.0	39.0	9.0	39.0	9.0	39.0	9.0	39.0	9.0	39.0	9.0	Total Split (s)	28.0	42.0	28.0	42.0	38.1	38.1	38.1	38.1	38.1	38.1	38.1		
Total Split (%)	25.9%	38.9%	25.9%	38.9%	25.9%	38.9%	25.9%	38.9%	25.9%	38.9%	25.9%	35.2%	35.2%	35.2%	35.2%	35.2%	35.2%	35.2%	35.2%	35.2%	35.2%	35.2%			
Yellow Time (s)	3.0	5.0	3.0	5.0	3.0	5.0	3.0	5.0	4.1	4.1	4.1	4.1	All-Red Time (s)	0.0	2.0	0.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	1.0	-3.0	1.0	-3.0	1.0	-3.0	1.0	-3.0	-2.1	-2.1	-2.1	-2.1	Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lead/Lag Optimize?	None	Min	Min	None	None	None	None	None	None	None	None		
Recall Mode	Act. Effct Green (s)	39.7	36.7	31.3	25.9	18.3	18.3	18.3	18.3	18.3	18.3	Act. Effct Green (s)	0.60	0.55	0.47	0.39	0.28	0.28	0.28	0.28	0.28	0.28	0.28		
Actuated G/C Ratio	0.38	0.30	0.08	0.55	0.13	0.12	0.13	0.12	0.12	0.12	0.12	Vic Ratio	9.4	9.9	7.6	17.7	21.6	15.0	29.0	20.4	6.4	0.51	0.51		
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	Queue Delay	9.4	9.9	7.6	17.7	21.6	15.0	29.0	20.4	6.4	0.0	0.0		
Total Delay	9.4	9.9	7.6	17.7	21.6	15.0	29.0	20.4	29.0	20.4	29.0	LOS	A	A	B	C	B	C	C	C	A	A	A		
Approach Delay	9.8	17.2	17.2	16.9	15.6	15.6	15.6	15.6	15.6	15.6	15.6	Paradigm Transportation Solutions Limited	Synchro 10 Report	Page 1	Synchro 10 Report	Page 1	Synchro 10 Report	Page 1	Synchro 10 Report	Page 1	Synchro 10 Report	Page 1	Synchro 10 Report	Page 1	



### HCM Signalized Intersection Capacity Analysis

210371  
1: Helena St/Thompson Rd & Garrison Rd

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	144	456	39	33	512	126	38	65	31	157	101	282
Traffic Volume (vph)	144	456	39	33	512	126	38	65	31	157	101	282
Ideal Flow (vphol)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Losttime (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Firb. ped./pikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Firb. ped/pikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Firb.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Firb. Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1568	3218	1662	3180	1662	3040	1646	3137	1373	1373	1373	1373
Firb/Permitted	0.25	1.00	0.45	1.00	0.68	1.00	0.69	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	408	3218	750	3180	1194	3040	1188	3137	1373	1373	1373	1373
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	157	496	42	36	557	137	41	71	34	171	110	307
R/TOR Reduction (vph)	0	4	0	0	18	0	0	25	0	0	0	0
Lane Group Flow (vph)	157	534	0	36	676	0	41	80	0	171	110	82
Confil. Ped. (#/hr)	1	1	1	1	1	1	1	1	1	1	1	1
Heavy Vehicles (%)	6%	2%	2%	0%	1%	2%	0%	6%	0%	1%	6%	7%
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	perm	NA	perm	NA	perm	NA
Protected Phases	7	4	3	8	2	2	2	2	2	2	2	2
Permitted Phases	4											
Actuated Green, G (s)	38.6	33.5		27.0	24.9		16.0	16.0		16.0	16.0	
Effective Green, g (s)	37.6	36.5		25.0	27.9		18.1	18.1		18.1	18.1	
Actuated GC Ratio	0.56	0.54		0.37	0.41		0.27	0.27		0.27	0.27	
Clearance Time (s)	3.0	7.0		3.0	7.0		6.1	6.1		6.1	6.1	
Vehicle Extension (s)	2.5	5.0		2.5	5.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	392	1734		305	1310		319	812		317	838	
v/s Ratio Prot	0.06	0.17		0.00	0.21		0.03	0.04		0.04	0.04	
v/s Ratio Perm	0.16			0.04			0.03	c0.14		0.06		
vic Ratio	0.40	0.31		0.12	0.52		0.13	0.10		0.54	0.13	0.22
Uniform Delay, d <sup>1</sup>	8.4	8.6		13.8	14.9		18.8	18.7		21.2	18.8	19.3
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d <sup>2</sup>	0.5	0.2		0.1	0.7		0.2	0.1		1.8	0.1	0.3
Delay (s)	8.9	8.8		13.9	15.5		19.0	18.7		23.0	18.9	19.6
Level of Service	A	A		B	B		B	B		C	B	
Approach Delay (s)	8.9			15.5			18.8			20.5		
Approach LOS	A			B			B			C		

Intersection Summary

HCM 2000 Control Delay

HCM 2000 Volume to Capacity ratio

Actuated Cycle Length (s)

Intersection Capacity Utilization

Analysis Period (min)

c Critical Lane Group

Synchro 10 Report  
Page 3

210371  
2: Helena St & Washington Rd/Albany St

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBT
Lane Group												
Lane Configurations												
Traffic Volume (vph)												
Future Volume (vph)												
Ideal Flow (vphol)												
Lane Util. Factor												
Firb.												
Firb. Protected												
Satd. Flow (prot)												
Firb/Permitted												
Satd. Flow (perm)												
Link Distance (m)												
Travel Time (s)												
Peak Hour Factor												
Heavy Vehicles (%)												
Adj. Flow (vph)												
Shared Lane Traffic (%)												
Lane Group Flow (vph)												
Sign Control												
Intersection Summary												
Area Type - Other												
Control Type: Unsignalized												
Intersection Capacity Utilization 30.0%												
Analysis Period (min) 15												

Paradigm Transportation Solutions Limited  
Page 3

Synchro 10 Report  
Page 4

Paradigm Transportation Solutions Limited  
Page 3

HCM Unsignalized Intersection Capacity Analysis  
2: Helena St & Washington Rd/Albany St

210371  
Base Year PM Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	WBC	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												Stop
Sign Control	Stop	0	2	48	23	0	14	4	45	38	23	Stop
Traffic Volume (vph)	26	36	0	2	48	23	0	14	4	45	38	
Future Volume (vph)	26	36	0	2	48	23	0	14	4	45	38	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	28	39	0	2	52	25	0	15	4	49	41	25
Direction Lane #	EB1	WB1	NB1	SB1								
Volume Left (vph)	67	79	19	115								
Volume Right (vph)	28	2	0	49								
Hadj (s)	0.11	-0.14	0.06	-0.03								
Departure Headway (s)	4.4	4.1	4.4	4.2								
Degree Utilization, x	0.08	0.09	0.02	0.13								
Capacity (veh/h)	788	838	776	824								
Control Delay (s)	7.8	7.5	7.5	7.9								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay	7.7											
Level of Service	A											
Intersection Capacity Utilization	30.0%											
Analysis Period (min)	15											
ICU Level of Service	A											

Queuing and Blocking Report

210371  
Base Year PM Peak Hour

Intersection: 1: Helena St/Thompson Rd & Garrison Rd												
Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	TR	L	T	TR	L	T	TR	L	T	T
Maximum Queue (m)	37.4	43.5	36.1	15.2	58.2	51.7	22.0	20.0	23.1	46.5	37.1	14.2
Average Queue (m)	17.5	19.5	13.4	5.3	29.9	22.4	8.2	6.8	9.1	22.6	14.1	1.1
95th Queue (m)	30.7	34.8	26.8	13.2	48.4	41.6	18.9	15.8	39.1	28.7	7.7	
Link Distance (m)	212.3	212.3	212.3	212.3	289.8	289.8	77.0	77.0	355.9	355.9		
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (m)	120.0						110.0					
Storage Blk Time (%)												
Queuing Penalty (veh)	0						0					
Intersection: 2: Helena St & Washington Rd/Albany St												
Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	LTR	LTR	LTR	LTR	LTR	LTR						
Maximum Queue (m)	12.7	16.5	9.2	13.1								
Average Queue (m)	6.5	7.6	1.4	4.7								
95th Queue (m)	11.2	13.2	5.4	10.9								
Link Distance (m)	220.5	337.5	204.6	434.3								
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (m)												
Storage Blk Time (%)												
Queuing Penalty (veh)	0											
Zone Summary												
Zone wide Queuing Penalty	0											

## **Appendix D**

### **Background Traffic Operations**







HCM Unsignalized Intersection Capacity Analysis  
2: Helena St & Washington Rd/Albany St

210371  
2026 Background AM Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												Stop
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Volume (vph)	26	22	0	4	18	45	3	40	2	13	14	4
Future Volume (vph)	26	22	0	4	18	45	3	40	2	13	14	4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	28	24	0	4	20	49	3	43	2	14	15	4
Direction Lane #	EB1	WB1	NB1	SB1								
Volume Left (vph)	52	73	48	33								
Volume Right (vph)	0	49	2	4								
Hadj (s)	0.15	-0.28	0.06	0.12								
Departure Headway (s)	4.3	3.9	4.3	4.3								
Degree Utilization, x	0.06	0.08	0.06	0.04								
Capacity (veh/h)	815	909	811	802								
Control Delay (s)	7.6	7.2	7.5	7.5								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay	7.4											
Level of Service	A											
Intersection Capacity Utilization	22.5%											
Analysis Period (min)	15											
ICU Level of Service	A											

Queuing and Blocking Report

210371  
2026 Background AM Peak Hour

Intersection: 1: Helena St/Thompson Rd & Garrison Rd												
Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	TR	L	T	TR	L	T	TR	L	T	T
Maximum Queue (m)	45.8	28.3	26.3	9.0	39.9	29.6	26.4	24.1	27.8	26.6	3.0	3.0
Average Queue (m)	21.5	13.0	10.7	1.2	19.9	11.4	10.8	8.2	9.1	11.7	9.1	0.1
95th Queue (m)	36.8	24.3	21.5	6.1	33.9	22.8	23.5	19.5	19.8	23.6	21.1	2.1
Link Distance (m)	212.3	212.3	212.3	212.3	289.8	289.8	77.0	77.0	335.9	335.9		
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (m)	120.0											
Storage Blk Time (%)												
Queuing Penalty (veh)												
Intersection: 2: Helena St & Washington Rd/Albany St												
Movement	EB	WB	NB	SB	LTR	LTR	LTR	LTR	LTR	LTR	LTR	LTR
Directions Served					12.7	18.7	11.4	12.7				
Maximum Queue (m)					5.9	6.9	2.5	2.0				
Average Queue (m)					11.6	13.8	7.0	6.7				
95th Queue (m)					220.5	337.5	204.6	434.3				
Link Distance (m)												
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (m)												
Storage Blk Time (%)												
Queuing Penalty (veh)												
Zone Summary												
Zone wide Queuing Penalty	0											



HCM Signalized Intersection Capacity Analysis			210371			
1: Helena St/Thompson Rd & Garrison Rd			2026 Background PM Peak Hour			
Movement	EBL	EBT	EBR	WBL	WBT	
Lane Configurations	153	484	41	35	544	134
Traffic Volume (vph)	153	484	41	35	544	134
Future Volume (vph)	1750	1750	1750	1750	1750	1750
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0	4.0
Total Losttime (s)	0.95	1.00	0.95	1.00	0.95	1.00
Flrb. per/Perf.	1.00	1.00	1.00	1.00	1.00	1.00
Flrb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Fit	0.95	1.00	0.95	1.00	0.95	1.00
Fit Protected	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1568	3218	1662	3180	1662	3040
Fit Permitted	0.23	1.00	0.44	1.00	0.68	1.00
Satd. Flow (perm)	373	3218	765	3180	1186	3040
Peak-hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	166	526	45	38	591	146
RTOR Reduction (vph)	0	4	0	0	18	0
Lane Group Flow (vph)	166	567	0	38	719	0
Confil. Ped. (#/hr)	1	6%	1	1%	2%	0%
Heavy Vehicles (%)	2%	2%	0%	1%	2%	0%
Turn Type	pm+pt	NA	pm+pt	NA	Perm	NA
Protected Phases	7	4	3	8	2	2
Permitted Phases	4					6
Actuated Green, G (s)	39.7	33.4	29.0	25.7	16.9	16.9
Effective Green, g (s)	38.7	36.4	27.0	28.7	19.0	19.0
Actuated GC Ratio	0.56	0.52	0.39	0.41	0.27	0.27
Clearance Time (s)	3.0	7.0	3.0	7.0	6.1	6.1
Vehicle Extension (s)	2.5	5.0	2.5	5.0	3.0	3.0
Lane Grp Cap (vph)	378	1680	325	1309	323	828
v/s Ratio Prot	0.06	0.18	0.00	0.23	0.03	0.04
v/s Ratio Perm	0.18		0.04		c0.15	0.06
vic Ratio	0.44	0.34	0.12	0.55	0.13	0.14
Uniform Delay, d <sup>1</sup>	9.0	9.7	13.4	15.6	19.1	19.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d <sup>2</sup>	0.6	0.3	0.1	0.8	0.2	0.1
Delay (s)	9.6	9.9	13.5	16.4	19.3	19.0
Level of Service	A	A	B	B	B	C
Approach Delay (s)	9.8		16.3		19.1	21.0
Approach LOS	A		B		B	C

Intersection Summary		
HCM 2000 Control Delay	15.7	HCM 2000 Level of Service
HCM 2000 Volume to Capacity ratio	0.54	B
Actuated Cycle Length (s)	69.7	Sum of lost time (s)
Intersection Capacity Utilization	60.0%	ICU Level of Service
Analysis Period (min)	15	
c Critical Lane Group		

Lanes, Volumes, Timings			2026 Background PM Peak Hour		
2: Helena St & Washington Rd/Albany St					
Lane Group					
Lane Configurations				Lane Configurations	
Traffic Volume (vph)	153	484	41	Traffic Volume (vph)	27
Future Volume (vph)	153	484	41	Future Volume (vph)	37
Ideal Flow (vphpl)	1750	1750	1750	Ideal Flow (vphpl)	1750
Total Losttime (s)	4.0	4.0	4.0	Lane Util. Factor	1.00
Flrb. Util. Factor	1.00	1.00	1.00	Fit	0.957
Firb. ped/bikes	1.00	1.00	1.00	Fit Protected	0.979
Fit	1.00	1.00	1.00	Satd. Flow (prot)	0
Fit Protected	0.95	1.00	0.95	Fit Permitted	0.979
Satd. Flow	1568	3218	1662	Satd. Flow (perm)	0
Fit Permitted	0.23	1.00	0.44	Link Speed (km/h)	50
Satd. Flow (perm)	373	3218	765	Link Distance (m)	238.9
Peak-hour Factor, PHF	0.92	0.92	0.92	Travel Time (s)	25.6
Adj. Flow (vph)	166	526	45	Peak Hour Factor	0.92
RTOR Reduction (vph)	0	4	0	Heavy Vehicles (%)	0%
Lane Group Flow (vph)	166	567	0	Adj. Flow (vph)	3%
Confil. Ped. (#/hr)	1	6%	1	Shared Lane Traffic (%)	50%
Heavy Vehicles (%)	2%	2%	0%	Lane Group Flow (vph)	0%
Turn Type	pm+pt	NA	pm+pt	Sign Control	75%
Protected Phases	7	4	3	Stop	0%
Permitted Phases	4			Stop	0%
Actuated Green, G (s)	39.7	33.4	29.0		0%
Effective Green, g (s)	38.7	36.4	27.0		0%
Actuated GC Ratio	0.56	0.52	0.39		0%
Clearance Time (s)	3.0	7.0	3.0		0%
Vehicle Extension (s)	2.5	5.0	2.5		0%
Lane Grp Cap (vph)	378	1680	325		0%
v/s Ratio Prot	0.06	0.18	0.00		0%
v/s Ratio Perm	0.18		0.04		0%
vic Ratio	0.44	0.34	0.12		0%
Uniform Delay, d <sup>1</sup>	9.0	9.7	13.4		0%
Progression Factor	1.00	1.00	1.00		0%
Incremental Delay, d <sup>2</sup>	0.6	0.3	0.1		0%
Delay (s)	9.6	9.9	13.5		0%
Level of Service	A	A	B		0%
Approach Delay (s)	9.8		16.3		0%
Approach LOS	A		B		0%

Intersection Summary		
Lane Type: Other		
Control Type: Unsignalized		
Intersection Capacity Utilization 30.4%	ICU Level of Service A	
Analysis Period (min) 15		

HCM Unsignalized Intersection Capacity Analysis  
2: Helena St & Washington Rd/Albany St

210371  
2026 Background PM Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	WBC	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												Stop
Sign Control	Stop	0	2	50	24	0	15	4	47	39	24	Stop
Traffic Volume (vph)	27	37	0	2	50	24	0	15	4	47	39	24
Future Volume (vph)	27	37	0	2	50	24	0	15	4	47	39	24
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	29	40	0	2	54	26	0	16	4	51	42	26
Direction Lane #	EB1	WB1	NB1	SB1								
Volume Left (vph)	69	82	20	119								
Volume Right (vph)	0	26	4	26								
Hadj (s)	0.11	-0.14	0.06	-0.03								
Departure Headway (s)	4.4	4.1	4.4	4.2								
Degree Utilization, x	0.08	0.09	0.02	0.14								
Capacity (veh/h)	784	834	771	821								
Control Delay (s)	7.8	7.6	7.5	7.9								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay	7.8											
Level of Service	A											
Intersection Capacity Utilization	30.4%											
Analysis Period (min)	15											
ICU Level of Service							A					

Queuing and Blocking Report

210371  
2026 Background PM Peak Hour

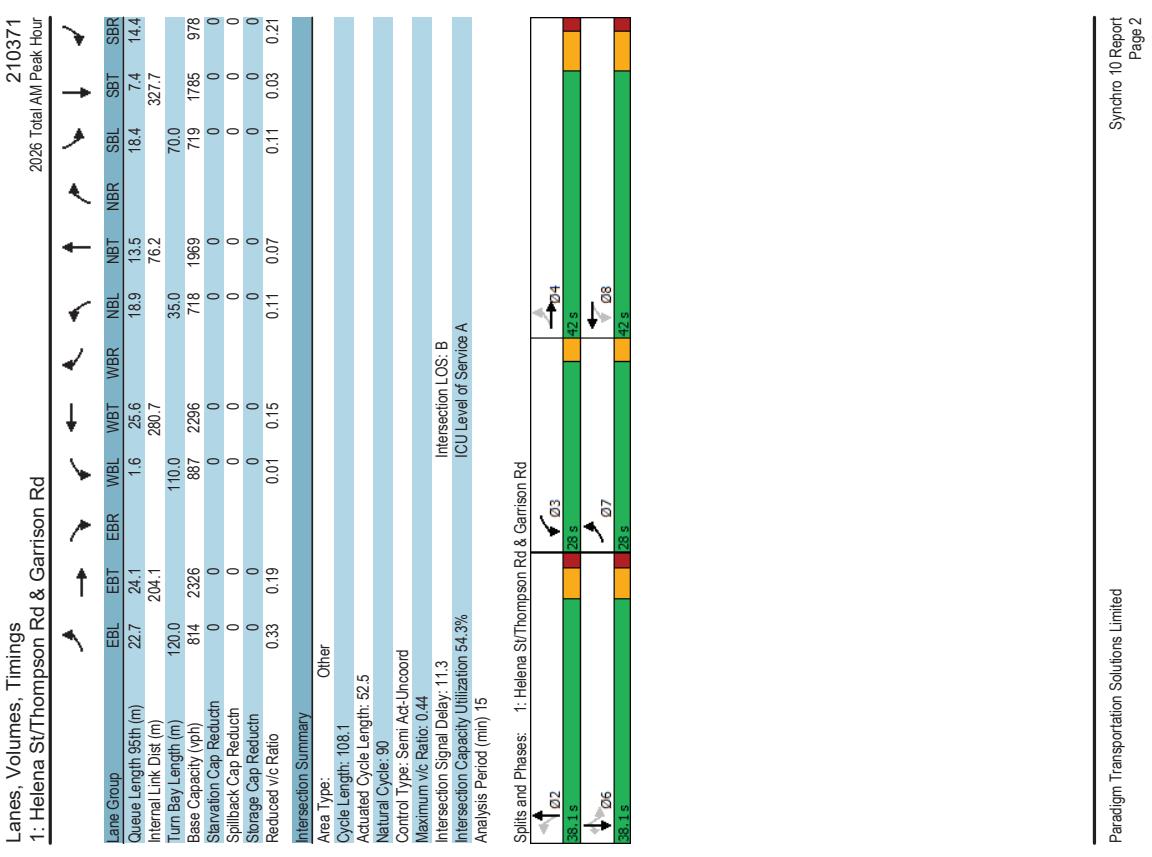
Intersection: 1: Helena St/Thompson Rd & Garrison Rd												
Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	TR	L	T	TR	L	T	TR	L	T	T
Maximum Queue (m)	34.1	39.6	34.3	58.2	51.9	21.8	21.5	22.8	49.0	35.7	12.4	12.4
Average Queue (m)	17.0	20.6	13.7	6.0	31.2	24.9	8.3	6.8	9.0	23.4	14.7	0.9
95th Queue (m)	29.4	34.4	27.0	14.2	49.6	44.1	18.7	16.3	40.8	28.5	6.7	6.7
Link Distance (m)	212.3	212.3	212.3	289.8	289.8	289.8	77.0	77.0	335.9	335.9		
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (m)	120.0											
Storage Blk Time (%)												
Queuing Penalty (veh)	0											
Intersection: 2: Helena St & Washington Rd/Albany St												
Movement	EB	WB	NB	SB	LTR	LTR	LTR	LTR	LTR	LTR	LTR	LTR
Directions Served					13.0	15.9	8.1	13.0				
Maximum Queue (m)												
Average Queue (m)					6.5	7.5	1.3	4.4				
95th Queue (m)					10.9	13.1	4.9	10.3				
Link Distance (m)					220.5	337.5	204.6	434.3				
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (m)												
Storage Blk Time (%)												
Queuing Penalty (veh)	0											
Zone Summary												
Zone wide Queuing Penalty	0											

## Appendix E

### Total Traffic Operations



Lanes, Volumes, Timings 1: Helena St/Thompson Rd & Garrison Rd												Lanes, Volumes, Timings 1: Helena St/Thompson Rd & Garrison Rd														
210371						210371						2026 Total AM Peak Hour						2026 Total AM Peak Hour								
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT			
Lane Configurations	244	376	28	7	255	56	73	118	16	70	56	187	22.7	24.1	1.6	25.6	18.9	13.5	18.4	7.4	14.4					
Traffic Volume (vph)	244	376	28	7	255	56	73	118	16	70	56	187	Internal Link Dist (m)	204.1	204.1	280.7	76.2	327.7								
Ideal Flow (vph)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	Turn Bay Length (m)	120.0	110.0	35.0	887	2296	718	1969	719	1785	978			
Storage Length (m)	1200	0	110.0	0.0	35.0	0.0	70.0	0.0	0.0	0.0	0.0	0.0	Stationary Cap Reductn	0	0	0	0	0	0	0	0	0	0			
Storage Lanes	1	0	1	0	1	0	1	0	1	0	1	1	Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0			
Taper Length (m)	7.5	0.95	0.95	1.00	0.95	0.95	1.00	0.95	0.95	1.00	0.95	1.00	Reduced v/c Ratio	0.33	0.19	0.01	0.15	0.11	0.07	0.11	0.03	0.21				
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	0.95	0.95	1.00	0.95	1.00	Fit Protected	0.850												
Fit	0.950	0.990			0.950	0.973		0.950	0.982		0.950		Intersection Summary													
Satd. Flow (prot)	1554	3158	0	1662	3111	0	1446	2975	0	1568	2703	1377	Area Type:	Other												
Fit Permitted	0.425	0.425		0.497		0.715		0.660		0.660			Cycle Length:	108.1												
Satd. Flow (perm)	655	3156	0	870	3111	0	1088	2975	0	1090	2703	1377	Actuated Cycle Length:	52.5												
Right Turn on Red			Yes			27	Yes						Natural Cycle:	90												
Satd. Flow (RTOR)		8											Control Type:	Semi Act-Uncoord												
Link Speed (km/h)	60				60		60		70		70		Maximum v/c Ratio:	0.44												
Link Distance (m)	228.1			304.7			304.7		100.2		100.2		Intersection Signal Delay:	11.3												
Travel Time (s)	13.7			18.3			18.3		5.2		5.2		Intersection Capacity Utilization:	54.3%												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	Analysis Period (min):	15												
Heavy Vehicles (%)	7%	3%	21%	0%	4%	4%	15%	10%	8%	6%	23%	8%														
Adj. Flow (vph)	265	409	30	8	277	61	79	128	17	76	61	203														
Shared Lane Traffic (%)																										
Lane Group Flow (vph)	265	439	0	8	336	0	79	145	0	76	61	203														
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	3	8	Perm	NA	Perm	NA	Perm														
Protected Phases	7	4			8		2		2		6		Permitted Phases	4			8		2		6		6			
Permitted Phases													Detector Phase	7	4	3	8	2	2	2	6	6	6			
Detector Phase													Switch Phase	7	4	Lead	Lag	Lead	Lag							
Switch Phase													Minimum Initial (s)	6.0	10.0	6.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		
Minimum Split (s)	9.0	39.0	9.0	39.0	39.0	39.0	38.1	38.1	38.1	38.1	38.1	38.1	Total Split (s)	28.0	42.0	28.0	42.0	38.1	38.1	38.1	38.1	38.1	38.1	38.1		
Total Split (%)	25.9%	38.9%	25.9%	38.9%	38.9%	38.9%	35.2%	35.2%	35.2%	35.2%	35.2%	35.2%	Yellow Time (s)	3.0	5.0	3.0	5.0	4.1	4.1	4.1	4.1	4.1	4.1	4.1		
AIR Red Time (s)	0.0	2.0	0.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	Lost Time Adjust (s)	1.0	-3.0	1.0	-3.0	-2.1	-2.1	-2.1	-2.1	-2.1	-2.1	-2.1		
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	Lead/Lag Optimize?	None	Min	None	None	None	None	None	None	None	None	None		
Lead/Lag													Recall Mode	Act Etc Green (s)	31.4	29.9	20.6	15.5	13.0	13.0	13.0	13.0	13.0	13.0	13.0	
Act Etc Green (s)	0.60	0.57	0.39	0.30	0.20	0.16	0.12	0.08	0.05	0.03	0.02	0.01	Actuated GC Ratio	0.44	0.24	0.02	0.36	0.25	0.25	0.25	0.25	0.25	0.25	0.25		
Actuated GC Ratio	0.44	0.24	0.02	0.36	0.25	0.16	0.08	0.04	0.02	0.01	0.00	0.00	Control Delay	7.5	6.6	6.1	15.3	21.5	16.6	21.3	17.6	17.6	17.6			
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	Queue Delay	7.5	6.6	6.1	15.3	21.5	16.6	21.3	17.6	17.6	17.6			
Queue Delay													Total Delay	A	A	A	B	C	C	B	A	B	A	B		
Total Delay													LOS													
Approach Delay	6.9												Approach LOS	A	A	15.1	18.4	18.4	18.4	11.8	11.8	11.8	11.8	11.8		
Approach LOS													Queue Length 50th (m)	10.0	8.2	0.4	12.1	6.2	5.2	6.0	2.4	0.0	0.0	0.0		





HCM Unsignalized Intersection Capacity Analysis									
2: Helena St & Washington Rd/Albany St									
Movement	EBL	E BT	EB R	W BL	W BT	W BR	N BL	N BT	N BR
Lane Configurations	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Sign Control	Stop	0	4	18	47	3	2	15	16
Traffic Volume (vph)	28	22	0	4	18	47	3	43	2
Future Volume (vph)	28	22	0	4	18	47	3	43	2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	30	24	0	4	20	51	3	47	2
Direction Lane #	EB 1	WB 1	NB 1	SB 1					
Volume Total (vph)	54	75	52	40					
Volume Left (vph)	30	4	3	16					
Volume Right (vph)	0	51	2	7					
Hadj (s)	0.15	-0.29	0.05	0.10					
Departure Headway (s)	4.3	3.9	4.3	4.3					
Degree Utilization, x	0.06	0.08	0.06	0.05					
Capacity (veh/h)	807	902	808	802					
Control Delay (s)	7.6	7.2	7.6	7.6					
Approach LOS	A	A	A	A					
Intersection Summary									
Delay	7.5								
Level of Service	A								
Intersection Capacity Utilization	23.9%								
Analysis Period (min)	15								
ICU Level of Service	A								

Lanes, Volumes, Timings									
2: Helena St & North Driveway									
Movement	EBL	E BT	EB R	W BL	W BT	W BR	N BL	N BT	N BR
Lane Configurations	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Sign Control	Stop	0	4	18	47	3	2	15	16
Traffic Volume (vph)	28	22	0	4	18	47	3	43	2
Future Volume (vph)	28	22	0	4	18	47	3	43	2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	30	24	0	4	20	51	3	47	2
Direction Lane #	EB 1	WB 1	NB 1	SB 1					
Volume Total (vph)	54	75	52	40					
Volume Left (vph)	30	4	3	16					
Volume Right (vph)	0	51	2	7					
Hadj (s)	0.15	-0.29	0.05	0.10					
Departure Headway (s)	4.3	3.9	4.3	4.3					
Degree Utilization, x	0.06	0.08	0.06	0.05					
Capacity (veh/h)	807	902	808	802					
Control Delay (s)	7.6	7.2	7.6	7.6					
Approach LOS	A	A	A	A					
Intersection Summary									
Delay	7.5								
Level of Service	A								
Intersection Capacity Utilization	23.9%								
Analysis Period (min)	15								
ICU Level of Service	A								

HCM Unsignalized Intersection Capacity Analysis							210371							
3: Helena St & North Driveway							2028 Total AM Peak Hour							
Movement	EBL	EBR	NBL	NBT	SBT	SBR	Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	W	W	3	3	188	90	3	W	W	3	4	175	90	3
Traffic Volume (veh/h)	20	3	3	188	90	3	Traffic Volume (vph)	16	3	4	175	90	3	
Future Volume (Veh/h)	20	3	3	188	90	3	Future Volume (vph)	16	3	4	1750	1750	1750	
Sign Control	Stop			Free	Free		Ideal Flow (vph)	1750	1750	1750	1750	1750	1750	
Grade	0%			0%	0%		Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	Fit	0.980						
Hourly flow rate (vph)	22	3	3	204	98	3	Fit Protected	0.959						
Pedestrians							Std. Flow (prot)	1612	0	0	1714	1709	0	
Lane Width (m)							Fit Permitted	0.959						
Walking Speed (m/s)							Std. Flow (perm)	1612	0	0	1714	1709	0	
Percent Blockage							Link Speed (km/h)	50						
Right turn flare (veh)							Link Distance (m)	1727						
Median type							Travel Time (s)	124						
Median storage veh							Peak Hour Factor	0.92						
Upstream signal (m)							Adj. Flow (vph)	17						
pX, platoon unblocked							Shared Lane Traffic (%)							
vC, conflicting volume							Lane Group Flow (vph)	20						
VC1, stage 1 conf vol							Sign Control	0						
VC2, stage 2 conf vol							Intersection Summary							
VCu, unblocked vol							Area Type:							
IC, single (s)							Other							
IC, 2 stage (s)							Control Type: Unsignedized							
If (s)	3.5	3.3	2.2				Intersection Capacity Utilization 23.5%							
p0 queue free %	97	100	100				Analysis Period (min)	15						
cLM capacity (veh/h)	681	956	1491				ICU Level of Service A							
Direction, Lane #		EB 1	NB 1	SB 1										
Volume Total	25	207	101											
Volume Left	22	3	0											
Volume Right	3	0	3											
cSH	706	1491	1700											
Volume to Capacity	0.04	0.00	0.06											
Queue Length 95th (m)	0.9	0.0	0.0											
Control Delay (s)	10.3	0.1	0.0											
Lane LOS	B	A												
Approach LOS	10.3	0.1	0.0											
Intersection Summary														
Average Delay														
Intersection Capacity Utilization														
Analysis Period (min)														

Lanes, Volumes, Timings							2026 Total AM Peak Hour						
3: Helena St & South Driveway							4: Helena St & South Driveway						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W	W	3	3	188	90	3	W	W	3	4	175	90
Traffic Volume (veh/h)	20	3	3	188	90	3	Traffic Volume (vph)	16	3	4	175	90	3
Future Volume (Veh/h)	20	3	3	188	90	3	Ideal Flow (vph)	1750	1750	1750	1750	1750	1750
Sign Control	Stop			Free	Free		Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Grade	0%			0%	0%		Fit	0.980					
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	Fit Protected	0.959					
Hourly flow rate (vph)	22	3	3	204	98	3	Std. Flow (prot)	1612	0	0	1714	1709	0
Pedestrians							Fit Permitted	0.959					
Lane Width (m)							Std. Flow (perm)	1612	0	0	1714	1709	0
Walking Speed (m/s)							Link Speed (km/h)	50					
Percent Blockage							Link Distance (m)	1727					
Right turn flare (veh)							Travel Time (s)	124					
Median type							Peak Hour Factor	0.92					
Median storage veh							Adj. Flow (vph)	17					
Upstream signal (m)							Shared Lane Traffic (%)						
pX, platoon unblocked							Lane Group Flow (vph)	20					
vC, conflicting volume							Sign Control	0					
VC1, stage 1 conf vol							Intersection Summary						
VC2, stage 2 conf vol							Area Type:						
VCu, unblocked vol							Other						
IC, single (s)							Control Type: Unsignedized						
IC, 2 stage (s)							Intersection Capacity Utilization 23.5%						
If (s)	3.5	3.3	2.2				Analysis Period (min)	15					
p0 queue free %	97	100	100				ICU Level of Service A						
cLM capacity (veh/h)	681	956	1491										
Direction, Lane #		EB 1	NB 1	SB 1									
Volume Total	25	207	101										
Volume Left	22	3	0										
Volume Right	3	0	3										
cSH	706	1491	1700										
Volume to Capacity	0.04	0.00	0.06										
Queue Length 95th (m)	0.9	0.0	0.0										
Control Delay (s)	10.3	0.1	0.0										
Lane LOS	B	A											
Approach LOS	10.3	0.1	0.0										
Intersection Summary													
Average Delay													
Intersection Capacity Utilization													
Analysis Period (min)													

HCM Unsignalized Intersection Capacity Analysis  
4: Helena St & South Driveway

210371  
2026 Total AM Peak Hour

Queuing and Blocking Report

4: Helena St/Thompson Rd & Garrison Rd  
210371  
2026 Total AM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	16	3	4	175	90	3	16
Traffic Volume (veh/h)	16	3	4	175	90	3	16
Future Volume (veh/h)				Free	Free		
Sign Control	Stop						
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	17	3	4	190	98	3	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None		None				
Median storage (veh)							
Upstream signal (m)							
pX <sub>c</sub> platoon unblocked							
vC <sub>c</sub> conflicting volume							
VC1 stage 1 conf vol	298	100	101				
VC2 stage 2 conf vol							
YC <sub>c</sub> , unblocked vol							
IC: single (s)	6.4	6.2	4.1				
IC, 2 stage (s)	3.5	3.3	2.2				
IF (s)							
p0 queue free %	98	100	100				
clM capacity (veh/h)	692	956	1491				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	20	194	101				
Volume Left	17	4	0				
Volume Right	3	0	3				
cSH	722	1491	1700				
Volume to Capacity	0.03	0.00	0.06				
Queue Length 95th (m)	0.7	0.1	0.0				
Control Delay (s)	10.1	0.2	0.0				
Lane LOS	B	A					
Approach Delay (s)	10.1	0.2	0.0				
Approach LOS	B						
Intersection Summary							
Average Delay		0.8					
Intersection Capacity Utilization		23.5%		ICU Level of Service	A		
Analysis Period (min)		15					

Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	16	3	4	175	90	3	16
Traffic Volume (veh/h)	16	3	4	175	90	3	16
Future Volume (veh/h)				Free	Free		
Sign Control	Stop						
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	17	3	4	190	98	3	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None		None				
Median storage (veh)							
Upstream signal (m)							
pX <sub>c</sub> platoon unblocked							
vC <sub>c</sub> conflicting volume							
VC1 stage 1 conf vol	298	100	101				
VC2 stage 2 conf vol							
YC <sub>c</sub> , unblocked vol							
IC: single (s)	6.4	6.2	4.1				
IC, 2 stage (s)	3.5	3.3	2.2				
IF (s)							
p0 queue free %	98	100	100				
clM capacity (veh/h)	692	956	1491				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	20	194	101				
Volume Left	17	4	0				
Volume Right	3	0	3				
cSH	722	1491	1700				
Volume to Capacity	0.03	0.00	0.06				
Queue Length 95th (m)	0.7	0.1	0.0				
Control Delay (s)	10.1	0.2	0.0				
Lane LOS	B	A					
Approach Delay (s)	10.1	0.2	0.0				
Approach LOS	B						
Intersection Summary							
Average Delay		0.8					
Intersection Capacity Utilization		23.5%		ICU Level of Service	A		
Analysis Period (min)		15					

## Queuing and Blocking Report

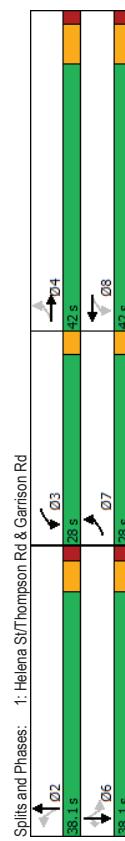
210371

Lanes, Volumes, Timings  
1: Helena St/Thompson Rd & Garrison Rd  
2026 Total AM Peak Hour

Intersection: 4: Helena St & South Driveway			
Movement	EB	NB	
Directions Served	LR	LT	
Maximum Queue (m)	9.2	2.6	
Average Queue (m)	4.1	0.1	
95th Queue (m)	11.4	1.5	
Link Distance (m)	164.0	434.3	
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (m)			
Storage Blk Time (%)			
Queuing Penalty (veh)			
<b>Network Summary</b>			
Network wide Queuing Penalty: 0			

Lane Group		EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations													
Traffic Volume (vph)		153	484	47	544	134	44	75	37	166	127	300	↑↑
Future Volume (vph)		153	484	47	544	134	44	75	37	166	127	300	↑↑
Ideal Flow (vphpl)		1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (m)		120.0	0.0	110.0	0.0	35.0	0.0	70.0	0.0	70.0	0.0	70.0	0.0
Storage Lanes		1	0	1	0	1	0	1	0	1	0	1	1
Taper Length (m)		7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util Factor		1.00	0.95	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Ped Bike Factor		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99
Fit		0.987	0.987	0.987	0.987	0.987	0.987	0.987	0.987	0.987	0.987	0.987	0.989
Fit Protected		0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Snd. Flow (prot)		1568	3214	0	1662	3179	0	1662	3039	0	1646	3137	1390
Fit Permitted		0.226	0.226	0.226	0.235	0.235	0.235	0.235	0.235	0.235	0.235	0.235	0.235
Snd. Flow (perm)		373	3214	0	761	3179	0	1161	3039	0	1170	3137	1372
Right Turn on Red			Yes										
Snd. Flow (R/T/R)		10		31		31		40		40		40	326
Link Speed (k/h)		60		60		60		70		70		70	70
Link Distance (m)		228.1		228.1		304.7		100.2		100.2		351.7	
Travel Time (s)		13.7		13.7		18.3		5.2		5.2		18.1	
Confli. Peds. (#/hr)		1		1		1		1		1		1	
Peak-Hour Factor		0.92		0.92		0.92		0.92		0.92		0.92	
Heavy Vehicles (%)		6%		2%		0%		1%		2%		0%	
Adj. Flow (vph)		166		526		51		45		591		146	
Shared Lane Traffic (%)		166		577		0		45		737		0	
Lane Group Flow (vph)		pm+pt		NA		pm+pt		NA		pm		NA	
Protected Phases		7		4		3		8		8		2	
Permitted Phases		4		4		8		8		2		6	
Detector Phase		7		4		3		8		2		2	
Switch Phase												6	6
Minimum Initial (s)		6.0		10.0		6.0		10.0		10.0		10.0	
Minimum Split (s)		9.0		39.0		9.0		39.0		38.1		38.1	
Total Split (s)		28.0		42.0		28.0		42.0		38.1		38.1	
Total Split (%)		25.9%		38.9%		25.9%		38.9%		35.2%		35.2%	
Yellow Time (s)		3.0		5.0		3.0		5.0		4.1		4.1	
All-Red Time (s)		0.0		2.0		0.0		2.0		2.0		2.0	
Lost Time Adjust (s)		1.0		-3.0		1.0		-3.0		-2.1		-2.1	
Total Lost Time (s)		4.0		4.0		4.0		4.0		4.0		4.0	
Lead/Lag		Lead		Lag		Lead		Lag					
Lead/Lag Optimize?		None		Min		Min		Min		None		None	
Recall Mode													
Act. Eff. Green (s)		41.5		36.5		32.9		27.4		19.3		19.3	
Actuated g/C Ratio		0.60		0.53		0.48		0.40		0.28		0.28	
Vc Ratio		0.42		0.34		0.10		0.58		0.15		0.16	
Control Delay		10.4		11.6		8.0		18.7		22.4		15.3	
Queue Delay		0.0		0.0		0.0		0.0		0.0		0.0	
Total Delay		10.4		11.6		8.0		18.7		22.4		15.3	
LOS		B		B		A		B		C		C	
Approach Delay				11.3		18.1		17.3					16.3

Lanes, Volumes, Timings 1: Helena St/Thompson Rd & Garrison Rd									
210371 2026 Total PM Peak Hour				2026 Total PM Peak Hour					
Lane Group	EBL	EBC	EBR	WBL	WBR	NBL	NBR		
Approach LOS	B	B	B	B	B	B	B		
Queue Length 50th (m)	8.4	23.7	21	36.1	4.7	4.2	20.1	7.1	0.0
Queue Length 95th (m)	22.5	46.1	76	72.3	15.3	12.4	49.0	17.1	19.6
Internal Link Dist (m)	204.1	204.1	280.7	110.0	35.0	76.2	327.7		
Turn Bay Length (m)	120.0	119.2	1854	603	1599	608	1630	869	
Base Capacity (vph)	660	1924	786	0	0	0	0	0	0
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.25	0.30	0.06	0.40	0.08	0.08	0.30	0.08	0.38
<b>Intersection Summary</b>									
Area 1 Type:	Other								
Cycle Length: 108.1									
Actuated Cycle Length: 69.2									
Natural Cycle: 90									
Control Type: Semi/Act-Uncoord									
Maximum v/c Ratio: 0.58									
Intersection Signal Delay: 15.4	Intersection LOS: B								
Intersection Capacity Utilization: 62.0%	ICU Level of Service B								
Analysis Period (min) 15									
<b>Splits and Phases:</b> 1: Helena St/Thompson Rd & Garrison Rd									
↓ 02	↑ 03	↓ 04	↑ 05	↓ 06	↑ 07	↓ 08	↑ 09		
38.1 s	28.5 s	42.5 s	28.5 s	38.1 s	28.5 s	42.5 s	28.5 s		



HCM Signalized Intersection Capacity Analysis									
1: Helena St/Thompson Rd & Garrison Rd									
2026 Total PM Peak Hour									
Movement	EBL	EBC	EBR	WBL	WBR	NBL	NBR	SBL	SBR
Lane Configurations									
Traffic Volume (vph)	153	484	47	41	544	134	44	75	37
Future Volume (vph)	153	484	47	41	544	134	44	75	37
Ideal Flow (vphol)	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util Factor									
Fpb, pb/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fpb, pd/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fit									
Fit Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Std. Flow (prot)	1568	3213	1662	3180	1662	3039	1646	3137	1373
Fit Permitted	0.23	1.00	0.43	1.00	0.66	1.00	0.67	1.00	1.00
Std. Flow (perm)	373	3213	761	3180	1162	3039	1169	3137	1373
Peak-hour Factor, DHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	166	526	51	45	591	146	48	82	40
RTOR Reduction (vph)	0	5	0	0	18	0	29	0	0
Lane Group Flow (vph)	166	572	0	45	719	0	48	33	0
Conf. Pers. (#/hr)	1	1	1	1	1	1	1	1	1
Heavy Vehicles (%)	6%	2%	2%	0%	1%	2%	0%	6%	0%
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	pm+pt	NA	pm+pt
Protected Phases	4	7	4	3	8	2	2	2	6
Permitted Phases	4	4	4	8	8	2	2	2	6
Actuated Green, G (s)	39.8	33.4	39.8	29.2	25.8	17.0	17.0	17.0	17.0
Effective Green, g (s)	38.8	36.4	38.8	27.2	28.8	19.1	19.1	19.1	19.1
Actuated g/C Ratio	0.56	0.52	0.56	0.39	0.41	0.27	0.27	0.27	0.27
Clearance Time (s)	3.0	7.0	3.0	7.0	3.0	6.1	6.1	6.1	6.1
Vehicle Extension (s)	2.5	5.0	2.5	5.0	3.0	3.0	3.0	3.0	3.0
Lane Grip Cap (vph)	378	1673	327	1310	317	830	319	857	375
vs Ratio Prot	0.06	0.18	0.06	0.23	0.03	0.03	0.03	0.04	
vs Ratio Perm	0.18	0.05	0.18	0.04	0.04	0.04	0.04	0.04	
vc Ratio	0.44	0.34	0.14	0.55	0.15	0.11	0.16	0.16	0.06
Uniform Delay, d1	9.0	9.8	13.4	15.6	19.3	19.0	21.8	19.3	19.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.6	0.3	0.1	0.8	0.2	0.1	2.3	0.1	0.3
Delay (s)	9.6	10.0	13.5	16.4	19.5	19.1	24.1	19.4	20.1
Level of Service	A	B	B	B	B	B	C	B	C
Approach Delay (s)	9.9	16.3	19.2	19.2	19.2	19.2	21.1	21.1	
Approach LOS	A	B	B	B	B	B	C	C	
Intersection Summary									
HCM 2000 Control Delay									
HCM 2000 Volume to Capacity ratio	0.54								
Actuated Cycle Length (s)									
Intersection Capacity Utilization	69.9								
Analysis Period (min)	15								
C Critical Lane Group									

Lanes, Volumes, Timings 2: Helena St & Washington Rd/Albany St										
210371 2026 Total PM Peak Hour										
Lane Group	EBL	EBT	EVR	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Configurations	31	37	0	2	50	28	0	18	4	52
Traffic Volume (vph)	31	37	0	2	50	28	0	18	4	52
Future Volume (vph)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fit										
Fit Projected	0.978	0	1684	0	0	1627	0	0	1554	0
Satd. Flow (prot)	0	1684	0	0	1627	0	0	1554	0	0
Fit Permitted	0.978	0	1684	0	0	1627	0	0	1646	0
Satd. Flow (perm)	0	1684	0	0	1627	0	0	1646	0	0
Link Speed (kh)	50		50		50		50		50	
Link Distance (m)	238.9		355.9		223.0		459.6			
Travel Time (s)										
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	0%	50%	2%	0%	7%	25%	0%	3%
Adj. Flow (vph)	34	40	0	2	54	30	0	20	4	57
Shared Lane Traffic (%)										
Lane Group Flow (vph)	0	74	0	0	86	0	0	24	0	132
Sign Control		Stop		Stop		Stop		Stop		
Intersection Summary										
Area Type:	Other									
Control Type: Unsignalized										
Intersection Capacity Utilization 31.3%										
Analysis Period (min)	15									

HCM Unsigned Intersections Capacity Analysis 2: Helena St & Washington Rd/Albany St										
210371 2026 Total PM Peak Hour										
Movement	EBL	EBT	EVR	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Configurations	31	37	0	2	50	28	0	18	4	52
Traffic Volume (vph)	31	37	0	2	50	28	0	18	4	52
Future Volume (vph)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fit										
Fit Projected	0.978	0	1684	0	0	1627	0	0	1554	0
Satd. Flow (prot)	0	1684	0	0	1627	0	0	1554	0	0
Fit Permitted	0.978	0	1684	0	0	1627	0	0	1646	0
Satd. Flow (perm)	0	1684	0	0	1627	0	0	1646	0	0
Link Speed (kh)	50		50		50		50		50	
Link Distance (m)	238.9		355.9		223.0		459.6			
Travel Time (s)										
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	0%	50%	2%	0%	7%	25%	0%	3%
Adj. Flow (vph)	34	40	0	2	54	30	0	20	4	57
Shared Lane Traffic (%)										
Lane Group Flow (vph)	0	74	0	0	86	0	0	24	0	132
Sign Control		Stop		Stop		Stop		Stop		
Intersection Summary										
Area Type:	Other									
Control Type: Unsignalized										
Intersection Capacity Utilization 31.3%										
Analysis Period (min)	15									

Lanes, Volumes, Timings 3: Helena St & North Driveway								210371 2026 Total PM Peak Hour							
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR									
Lane Configurations	W	5	5	150	200	16									
Traffic Volume (vph)	0	5	5	150	200	16									
Future Volume (vph)	0	5	5	150	200	16									
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750									
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00									
Fit	0.865		0.990												
Fit Protected			0.999												
Satd. Flow (prot)	1484	0	0	1714	1699	0									
Fit Permitted			0.999												
Satd. Flow (perm)	1484	0	0	1714	1699	0									
Link Speed (kh)	50		50	50											
Link Distance (m)	176.9		116.0	1195.0											
Travel Time (s)	12.7		8.4	86.0											
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92									
Adj. Flow (vph)	0	5	5	163	217	17									
Shared Lane Traffic (%)															
Lane Group n Flow (vph)	5	0	0	168	234	0									
Sign Control	Stop		Free	Free											
Intersection Summary							ICU Level of Service A								
Area 1 Type:	Other														
Control 1 Type:	Unsignalized														
Intersection Capacity Utilization	23.0%														
Analysis Period (min)	15														

HCM Unsignalized Intersection Capacity Analysis 3: Helena St & North Driveway								2026 Total PM Peak Hour							
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR									
Lane Configurations	W	5	5	150	200	16									
Traffic Volume (vph)	0	5	5	150	200	16									
Future Volume (vph)	0	5	5	150	200	16									
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750									
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00									
Fit	0.865		0.990												
Fit Protected			0.999												
Satd. Flow (prot)	1484	0	0	1714	1699	0									
Fit Permitted			0.999												
Satd. Flow (perm)	1484	0	0	1714	1699	0									
Link Speed (kh)	50		50	50											
Link Distance (m)	176.9		116.0	1195.0											
Travel Time (s)	12.7		8.4	86.0											
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92									
Adj. Flow (vph)	0	5	5	163	217	17									
Shared Lane Traffic (%)															
Lane Group n Flow (vph)	5	0	0	168	234	0									
Sign Control	Stop		Free	Free											
Intersection Summary							ICU Level of Service A								
Area 1 Type:	Other														
Control 1 Type:	Unsignalized														
Intersection Capacity Utilization	23.0%														
Analysis Period (min)	15														

Lanes, Volumes, Timings  
4: Helena St & South Driveway

HCM Unsigneded Intersection Capacity Analysis  
4: Helena St & South Driveway

	2026 Total PM Peak Hour				2026 Total PM Peak Hour							
	EBL	EBR	NBL	NBT	SBT	SBR	EBL	EBR	NBL	NBT	SBT	SBR
Lane Group												
Lane Configurations	W	7	6	6	148	190	15	7	6	6	148	15
Traffic Volume (vph)		7	6	6	148	190	15	7	6	6	148	15
Future Volume (vph)		1750	1750	1750	1750	1750	1750	Stop	Free	Free	Free	Free
Peak Flow (vphpl)		1750	1750	1750	1750	1750	1750					
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	Grade	0%	0%	0%	0%
Fit	0.937		0.937		0.930		0.930	Peak-Hour Factor	0.92	0.92	0.92	0.92
Fit Protected	0.974		0.988					Hourly flow rate (vph)	8	7	7	207
Satd. Flow (prot)	1566	0	0	1712	1689	0		Pedestrians				16
Fit Permitted	0.974		0.988					Lane Width (m)				
Satd. Flow (perm)	1566	0	0	1712	1689	0		Walking Speed (m/s)				
Link Speed (km/h)	50		50	50				Percent Blockage				
Link Distance (m)	172.7		459.6	116.0				Right turn flare (veh)				
Travel Time (s)	12.4		33.1	8.4				Median type				
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		Median storage (veh)				
Adj. Flow (vph)	8	7	7	161	207	16		Upstream signal (m)				
Shared Lane Traffic (%)								PX, platoon unblocked				
Lane Group n Flow (vph)	15	0	0	168	223	0		vC, conflicting volume				
Sign Control	Stop		Free	Free				vC1, stage 1 conf vol				
Intersection Summary								vC2, stage 2 conf vol				
Area 1 Type:	Other							vCu, unblocked vol				
Control 1 Type:	Unsigneded							vC, single (s)				
Intersection Capacity Utilization	23.7%							vC, 2 stage (s)				
Analysis Period (min)	15							If (s)				
								p0 queue free %				
								M capacity (veh/h)				
								Direction Lane #				
								EB 1	NB 1	SB 1		
								Volume Total	15	168	223	
								Volume Left	8	7	0	
								Volume Right	7	0	16	
								cSH	685	1346	1700	
								Volume to Capacity	0.02	0.01	0.13	
								Queue Length 95th (m)	0.5	0.1	0.0	
								Control Delay (s)	10.3	0.4	0.0	
								Lane LOS	B	A		
								Approach Delay (s)	10.3	0.4	0.0	
								Approach LOS	B			
								Intersection Summary				
								Average Delay				
								Intersection Capacity Utilization				
								Analysis Period (min)				
									0.5	23.7%	ICU Level of Service	
									15		A	

### Queuing and Blocking Report

210371

Queuing and Blocking Report

210371

2026 Total PM Peak Hour

#### Intersection: 1: Helena St/Thompson Rd & Garrison Rd

2026 Total PM Peak Hour

Movement	EB	EB	WB	WB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	TR	L	T
Maximum Queue (m)	40.9	44.0	39.4	16.7	53.6	52.9	23.2	48.4
Average Queue (m)	18.3	21.6	15.5	6.2	32.1	25.6	8.5	7.8
95th Queue (m)	33.2	37.3	30.4	14.6	49.9	45.4	18.5	17.4
Link Distance (m)	212.3	212.3	212.3	212.3	289.8	289.8	77.0	77.0
Upstream Blk Time (%)								
Queuing Penalty (veh)								
Storage Bay Dist (m)	120.0		110.0		35.0		0	0
Storage Blk Time (%)							0	0
Queuing Penalty (veh)							0	0

#### Intersection: 2: Helena St & Washington Rd/Albany St

2026 Total PM Peak Hour

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (m)	13.7	15.3	9.6	14.5
Average Queue (m)	6.7	7.5	1.6	5.6
95th Queue (m)	10.8	12.3	5.4	12.5
Link Distance (m)	220.5	337.5	204.6	434.3
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (m)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

#### Intersection: 3: Helena St & North Driveway

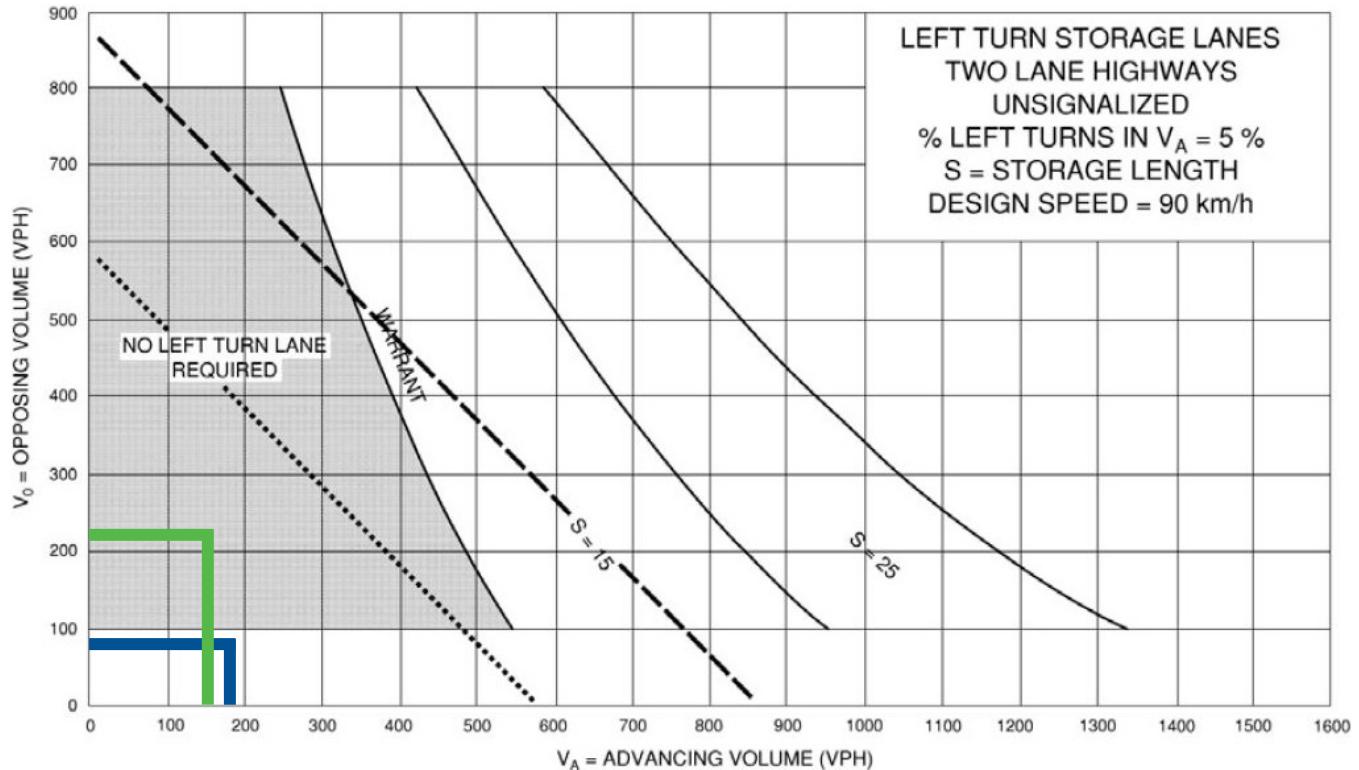
2026 Total PM Peak Hour

Movement	EB	NB
Directions Served	LR	LT
Maximum Queue (m)	8.9	5.7
Average Queue (m)	1.2	0.3
95th Queue (m)	6.0	3.4
Link Distance (m)	168.4	99.2
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (m)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

## Appendix F

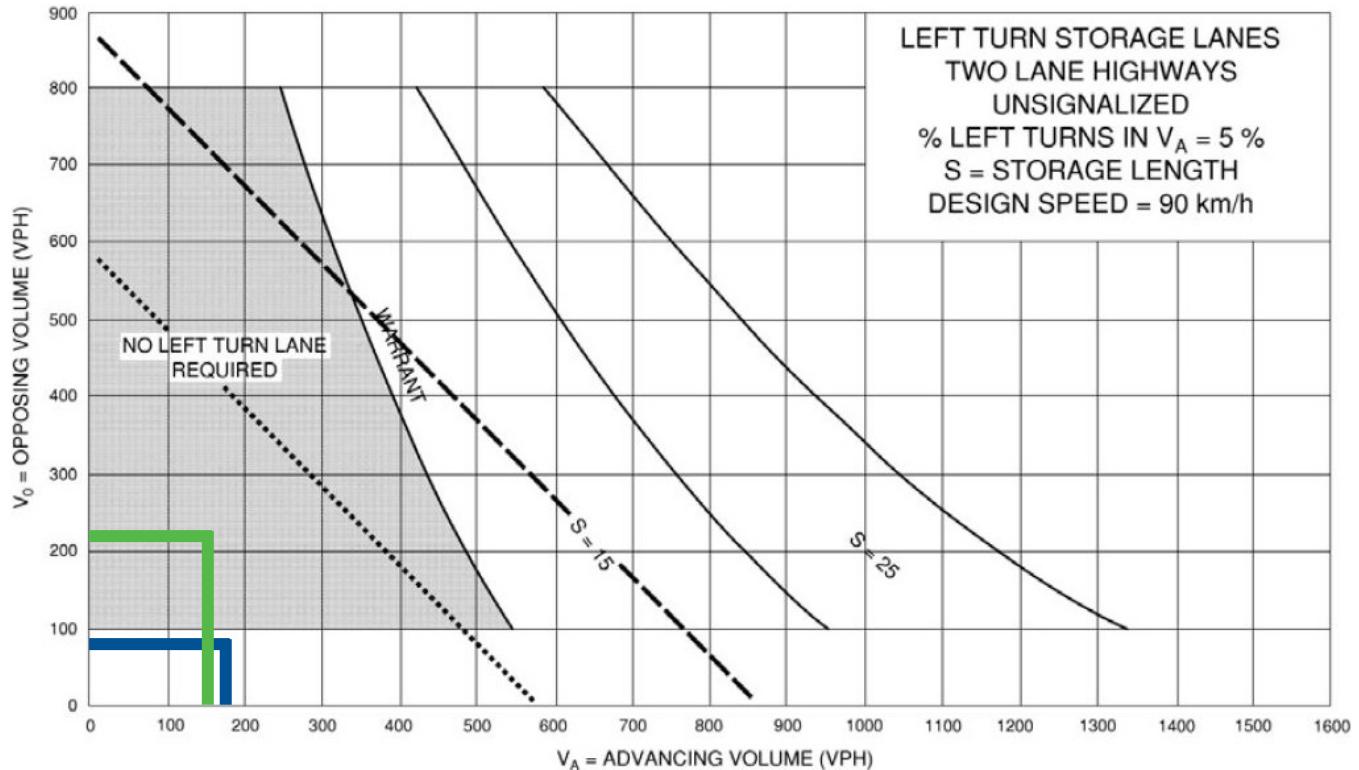
### Left-Turn Lane Warrants





613 Helena Street TIS Update  
210371

Left-Turn Lane Warrant  
Northbound Helena St at North  
Driveway - 2026 Total Conditions



— AM Peak Hour  
— PM Peak Hour

## Left-Turn Lane Warrant Northbound Helena St at South Driveway - 2026 Total Conditions

